

SITE INVESTIGATION FACTUAL REPORT

Report No: SI-555697
Client: Sedgwick International UK - Maidstone
Site: Flat 10, 38 Colney Hatch Lane
Haringey
Client Ref: 9646135
Date of Visit: 25/01/2023

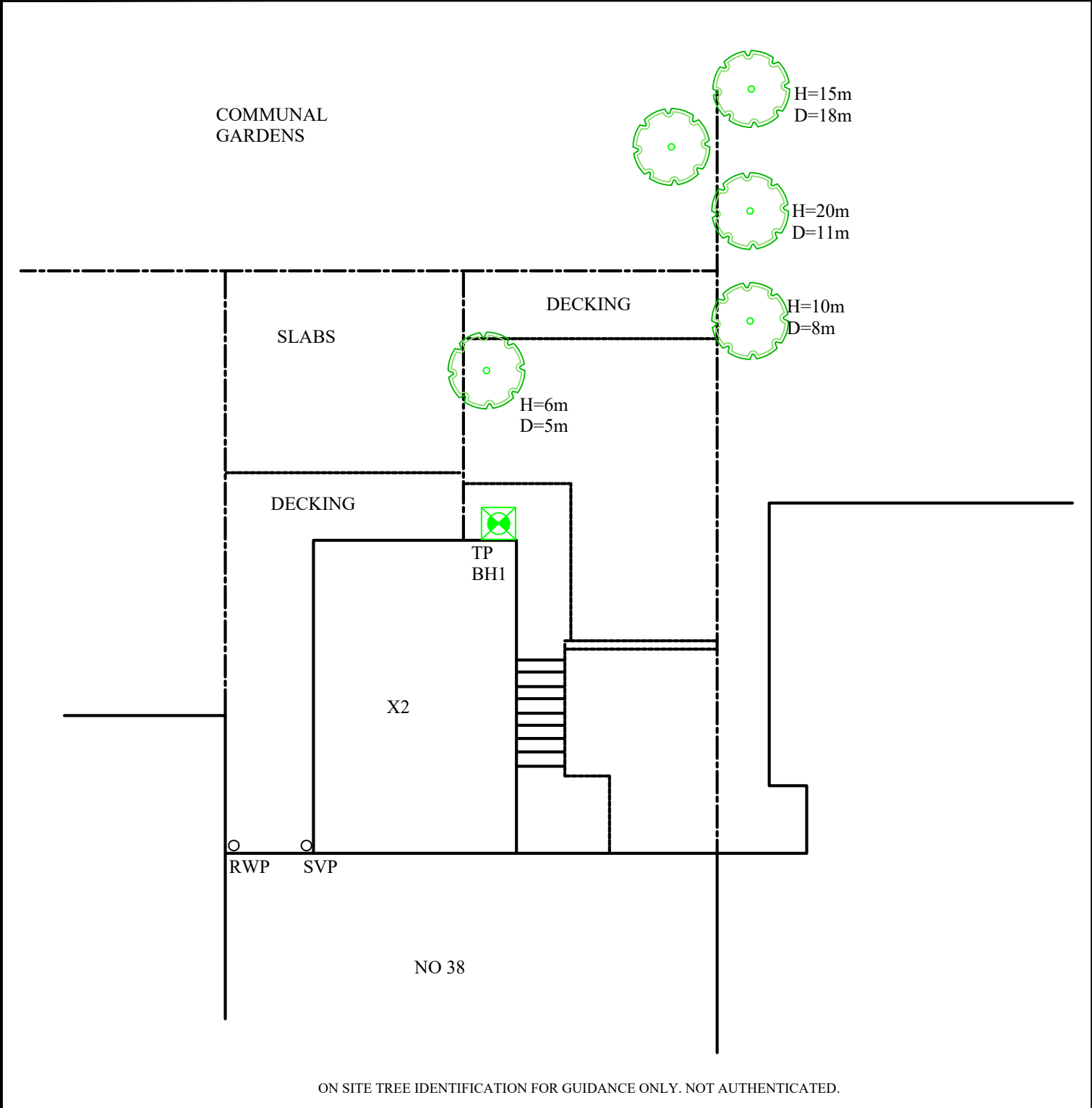


Home Emergency Response - Subsidence Investigation - Drainage Services – Crack & Level Monitoring – Property Video Surveys

Unit E2 First Floor Suite, Boundary Court
Willow Farm Business Park, Castle Donington
Leicestershire, DE74 2NN

☎ 0843 2272362
✉ enquiries@cet-uk.com
💻 www.cet-uk.com

CET is the trading name of CET Structures Ltd
Registered in England No. 02527130



Scale: N.T.S.

TEST REPORT: Trial Pit

REPORT NUMBER: C1072923 / 233131.1.1.1

TRIAL PIT REF: TP1

CLIENT: Sedgwick International UK

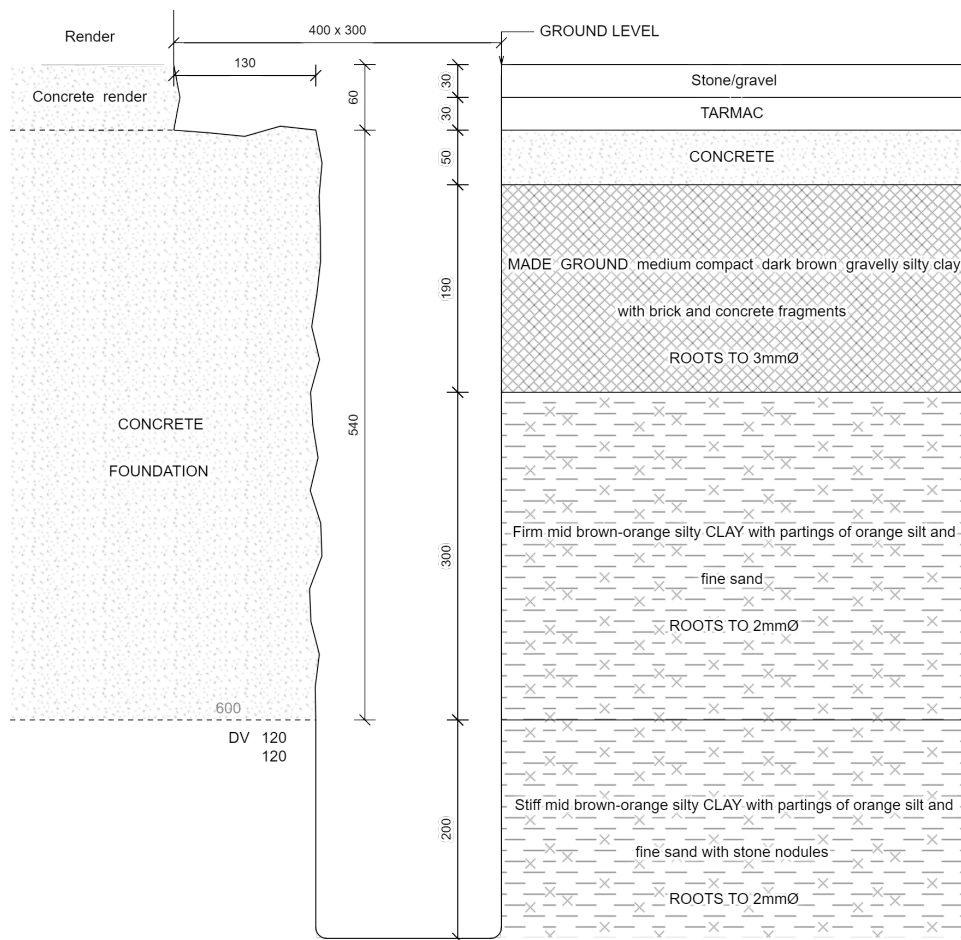
JOB NO: 555697

EXCAVATION METHOD: Hand tools

DATE: 25/01/2023

SITE: 38 COLNEY HATCH LANE

WEATHER: Dry



For Strata below 800mm see Bore Hole log

Key:

D Small disturbed sample J Jar sample
B Bulk disturbed sample V Pilcon vane (kPa)
W Water sample M Mackintosh probe
TDTD Too dense to drive

Remarks:

Test results reported relate only to the items tested.
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Opinions and interpretations expressed herein are outside the scope of UKAS Accreditation.
The laboratory does not apply a conformity statement to test reports as standard, unless specifically requested by the customer.

For and on behalf of CTS
Adam Mason - Quality Control



Approved Signatory
Report date 31-Jan-23

Borehole		1		Sheet:	1 of 1	Site:	38 COLNEY HATCH LANE			
				Job No:	555697					
				Date:	25/01/2023					
Boring Method:		Hand Auger		Ground Level:		Client:	SEDGWICK INTERNATIONAL UK			
Diameter (mm):		75	Weather:		dry					
Depth	Soil Description					Thickness	Legend	Samples and Tests		
(m)								Depth	Type	Result
0.00	See Trial Pit					0.80				
0.80	Stiff brown silty CLAY with partings of orange and brown silt and fine sand with stone nodules and claystone deposits					0.50	x — x			
							x — x			
							x — x	1.00	DV	120
							x — x			114
							x — x			
1.30	End of BH									
Remarks: Bh ends at 1.3m, obstruction thought to be claystone, too hard to hand auger. BH dry and open on completion.						Key: D - Disturbed Sample B - Bulk Sample W - Water Sample Roots J - Jar Sample Roots V - Pilcon Shear Vane (kPa Roots M - Mackintosh Probe Depth to Water (m) TDTD - Too Dense To Drive	To	Max		
							Depth	Dia		
							(m)	(mm)		
							1.30	1		
Logged:	MR	AM	Checked:	Approved:	Version	V1.0 28/01/16		N.T.S.		

SITE INVESTIGATION LABORATORY TEST REPORT

SI REPORT NUMBER: 555697

CLIENT : CET Property Assurance (Sedgwick International UK)

SITE:
Flat 10, 38 Colney Hatch Lane
London
N10 1DU

DATE OF SITE VISIT:
25/01/2023

DATE RECEIVED BY LABORATORY:
31/01/2023

Lesley Kirby
Compiled by :
L. Kirby - Laboratory Supervisor

Lesley Kirby
Approved by :
L. Kirby - Laboratory Supervisor

DATE REPORTED: 24-Feb-2023

Laboratory Summary Results

Our Ref : 555697

Date Sampled: 25/01/2023

Location : Flat 10, 38 Colney Hatch Lane, London N10 1DU

Date Received : 31/01/2023

Client: CET Property Assurance (Sedgwick International UK)

Date Tested : 15/02/2023

Address: Unit 4, Boundary Court, Willow Farm Business Park, Castle Donington, DE74 2NN

Date of Report : 24/02/2023

Sample Ref		Type	Moisture Content (%) [1]	Soil Fraction > 0.425mm (%) [2]	Liquid Limit (%) [3]	Plastic Limit (%) [4]	Plasticity Index (%) [5]	Liquidity * Index [5]	Modified * Plasticity Index (%) [6]	Soil * Class [7]	Filter Paper Contact Time (d)	Soil Sample Suction (kPa) [8]	Oedometer Strain [9]	Estimated * Heave Potential (Dd) (mm)[10]	In situ * Shear Vane Strength (kPa) [11]	Organic * Content (%) [12]	pH Value [13]	Sulphate Content		* Class [16]
TP/BH No	Depth (m)																	SO ₃ (g/l) * [14]	SO ₄ (mg/l) [15]	
1	U/S 0.60	D	35	<5	83	30	53	0.09	53	CV	7	372			120					
	1.0	D	29	<5	82	25	57	0.07	57	CV	7	685			117					

Test Methods / Notes

[1] BS 1377 : Part 2 : 1990, Test No 3.2

[2] Estimated if <5%, otherwise measured

[3] BS 1377 : Part 2 : 1990, Test No 4.4

[4] BS 1377 : Part 2 : 1990, Test No 5.3

[5] BS 1377 : Part 2 : 1990, Test No 5.4

[6] BRE Digest 240 : 1993

[7] BS 5930 : 2018 : Figure 8 - Plasticity Chart for the classification of fines soils

[8] Building Research Establishment Information Paper 4/93

[9] In Accordance with BS 1377-5 : 1990 : Clause 3

[10] Estimated Heave Potential (Dd)

[11] Values of shear strength were determined in situ by CTS using

a Pilcon hand vane or Geonor vane (GV).

[12] BS 1377 : Part 3 : 2018 + A1 2021 Clause 4 - Tested By CTS Leicester

[13] BS 1377 : Part 3 : 2018 + A1 2021 Clause 12 - Tested By CTS Leicester

[14] Sulphate content as SO₃ as required by BS1377 1990 has been provided for information purposes - Tested By CTS Leicester

[15] BS 1377 : Part 3 : 2018 + A1 2021 Clause 7.6 - Tested By CTS Leicester

[16] BRE Special Digest One (Concrete in Aggressive Ground) August 2005

Note that if the SO₄ content falls into the DS-4 or DS-5 class, it would be prudent to consider the sample as falling into the DS-4M or DS-5M class respectively unless water soluble magnesium testing is undertaken to prove otherwise.

PSD Chart - BS 1377: Part 2 : 1990, Test No 9.2

* These tests are not UKAS accredited

Full reports can be provided upon request.

Key

D Disturbed sample (small)
B Disturbed sample (bulk)
U Undisturbed sample
W Groundwater sample
ENP Essentially Non-Plastic by inspection
U/S Underside of Foundation



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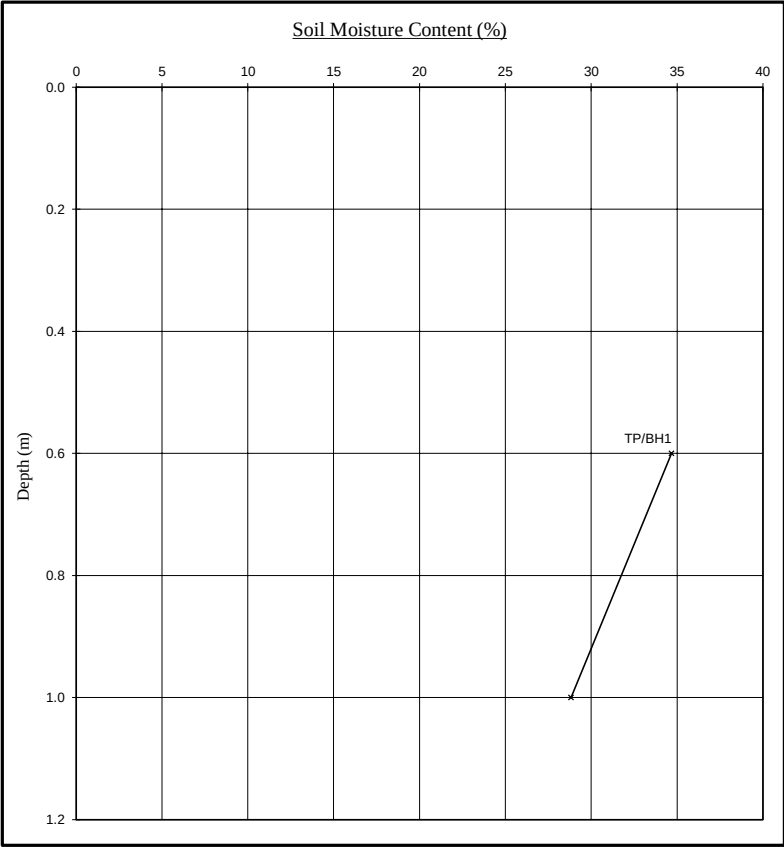
Construction Testing Solutions Ltd - Lawness Barns, Mountnessing Road, Billericay, Essex, CM12 0TS

Version: 5BH V3.7 - 21.02.2023

4161

Moisture Content Profiles

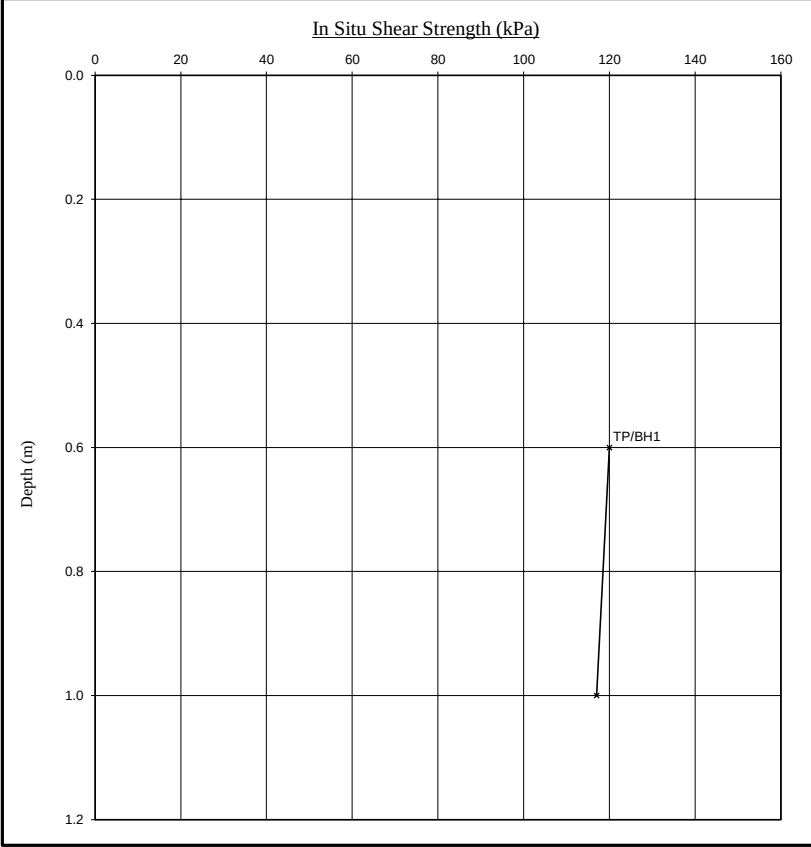
Our Ref : 555697
Location : Flat 10, 38 Colney Hatch Lane, London N10 1DU
Work carried out for: CET Property Assurance (Sedgwick International UK)



- Notes
1. If plotted, 0.4 LL and PL+2 (after Driscoll, 1983) should only be applied to London Clay (and similarly overconsolidated clay) at shallow depths.
 2. Unless specifically noted the profiles have not been related to a site datum.

Shear Strength Profiles

Date Sampled : 25/01/2023
Date Received : 31/01/2023
Date Tested : 15/02/2023
Date of Report : 24/02/2023

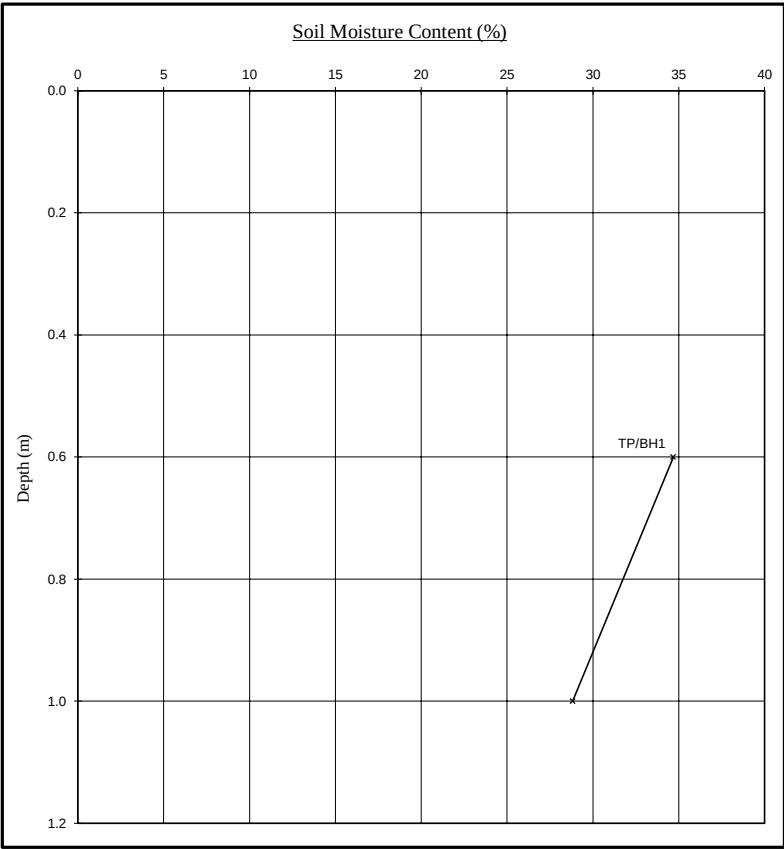


- Note
1. Unless otherwise stated, values of Shear Strength were determined in situ by CTS using a Pilcon Hand Vane the calibration of which is limited to a maximum reading of 120 kPa.
 2. Unless specifically noted the profiles have not been related to a site datum.

Moisture Content Profiles

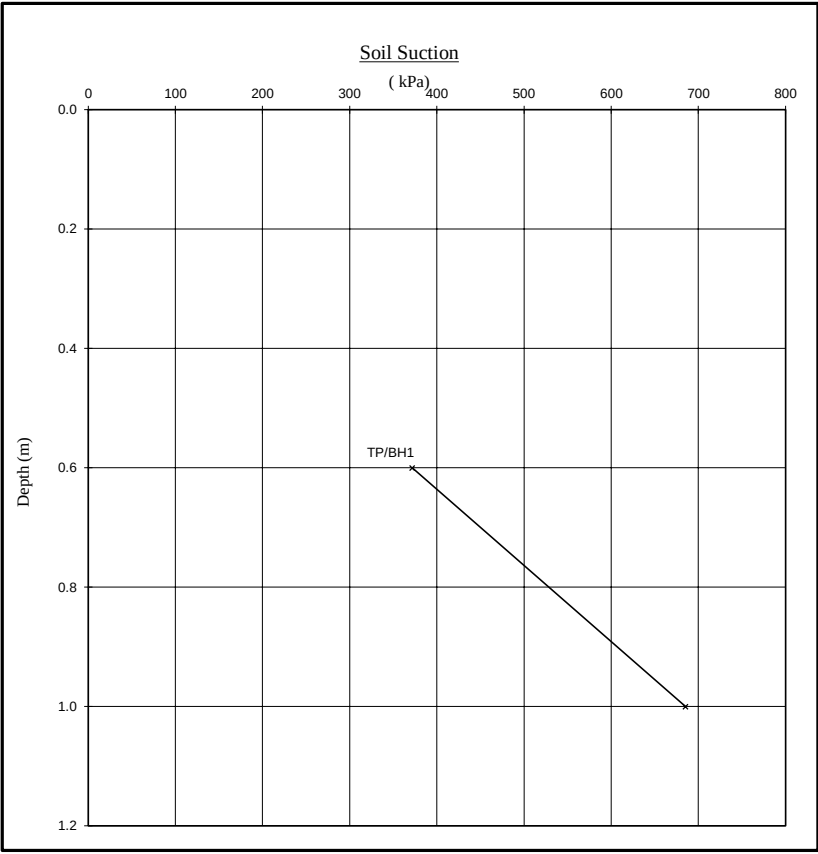
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- Notes
1. If plotted, 0.4 LL and PL+2 (after Driscoll, 1983) should only be applied to London Clay (and similarly overconsolidated clay) at shallow depths.
 2. Unless specifically noted the profiles have not been related to a site datum.

Soil Suction Profiles



Note

When shown, the theoretical equilibrium suction profiles are based on conventional assumptions associated with London Clay (and similarly overconsolidated clays) at shallow depths. Note that the sample disturbance component is dependant on the method of sampling and any subsequent recompaction. The above plots show this to be 100kPa which is the value suggested by the BRE on the basis of their limited number of tests on recompacted samples. This may or may not be appropriate in this instance and judgement should be exercised.

Construction Testing Solutions
4 Oak Spinney Park
Ratby Lane
Leicester Forest East
Leicestershire
LE3 3AW

Intec
Parc Menai, Bangor,
Gwynedd, North Wales
LL57 4FG
Tel: 01248 672652
Fax: 01248 672601

ROOT IDENTIFICATION

38 Colney Hatch Lane

Client Reference: 555697
Report Date: 3 February 2023
Our Ref: R50453

Sub Sample	Species Identified	Root Diameter	Starch
TP1:			
USF	<i>Salix</i> spp. *	1 1.5 mm	Moderate
BH1:			
to 1.3m	<i>Salix</i> spp. *	2 <1 mm	Low

Comments:

- 1 - Plus 3 others also identified as *Salix* spp.
- 2 - Plus 2 others also identified as *Salix* spp.

Salix spp. are willows.

* EPSL research has developed a unique ability to differentiate Willows from Poplars. No other laboratory in the UK can currently provide this service. We now offer this benefit at no extra cost.

Signed: G S Turner

Unless we are otherwise instructed in writing, the above sample material will normally be disposed of 6 years after the date of this report.

