

Seven

Basement waterproofing

7.1

The proposed basement will be designed to achieve a Grade 3 level of waterproofing protection as outlined in BS 8102:2009.

7.2

A reinforced concrete walls and basement slab will be cast using water resistant concrete to form the primary barrier with an internally drained cavity system as a secondary barrier against possible water ingress.

As part of the system any water that seeps through will be collected in a sump and be pumped up to high level where it will drain under gravity into the main drainage system.

Eight

Party wall matters

8.1

The proposed development works falls within the scope of the Party Walls Act 1996. Procedures under the Act will be dealt with in full by the Employer's Party Wall Surveyor. The Party Wall Surveyor will prepare and serve necessary Notices under the provisions of the Act and agree on Party Wall Awards in the event of disputes. The Contractor will be required to provide the Party Wall Surveyor with appropriate drawings, method statements and other relevant information covering the works that are notable under the Act. The resolution of matters under the Act and provisions of the Party Wall Awards will protect the interests of all owners.

8.2

The designs for the basement and refurbishment will be developed so as not to preclude or inhibit similar, or indeed any, works on the adjoining properties. This will be verified by the Surveyors as part of the process under the Act.

Nine

Hydrogeological statement summary

9.1

According to the British Geological Survey data, the site's underlying bedrock geology is London Clay. There are no superficial deposits recorded on the site.

There are no publicly available borehole records for the site itself nor its immediate surrounds. The nearest boreholes are along Cumberland Road, Winslow Place and Wood Common Green, and give the following information:

- Weathered London Clay is encountered as shallow as 0.55m below ground level
- Thanet Sand and Upper Chalk is encountered approx. 52m below ground level
- A water level was encountered in borehole TQ39SW651 near Winslow Place, at 7.0m below ground level.

The nearest watercourse to the site is New River, located approximately 0.33km to the south of the site and also 0.69km to the north of the site.

The Haringey SFRA maps show that New River is culverted between these two points and runs along the eastern side of the site, beneath Nightingale Gardens. The Haringey Surface Water Management Plan states that New River is classified as a canal and is owned and managed by Thames Water.

The site lies above a Source Protection Zone designated as Zone I – Inner Protection Zone.

While the London Clay Formation is classified as an 'Unproductive Stratum' it can still be water-bearing. The water pressures within the clay in the depths of current interest are likely to be hydrostatic, which means they increase linearly with depth. These can be modified by tree root activity or the influence of man-made changes such as utility trenches (which can act either as land drains or as sources of water and high groundwater pressures).

Any silt or sand partings, laminations or thicker beds are likely to contain free groundwater, and where these are laterally continuous, they can give rise to moderate water entries into excavations.

In most cases there will be only very limited or no natural flow in these silt/sand horizons.

A site-specific hydrogeological investigation will be carried out and presented for planning submission.

9.2

The impact of subterranean development on groundwater flows is negligible as groundwater flows will find an alternative route if blocked by a subterranean structure.

9.3

A Flood Risk Assessment and SUDS Report will be produced by Elliott Wood to demonstrate that the proposed works should not increase the risk of flooding in the area.

Ten

Monitoring during excavation and construction

10.1

The Contractor shall provide monitoring to all structures and infrastructure adjacent to the basement excavation at the time of excavation and construction.

10.2

Monitoring shall be completed as follows:

- 1) One month prior to any works being started to provide a base reading.
- 2) On a weekly basis during the excavation and until the basement slab and lining wall has been cast.
- 3) On a monthly basis thereafter for a 6-month period following completion of the notifiable works.

10.3

Cumulative movement of survey points must not exceed:

- a. Settlement
Code amber trigger values: +/-4mm
Code red trigger values: +/-8mm
- b. Lateral displacement
Code amber trigger values: +/-4mm
Code red trigger values: +/-8mm

10.4 Movement approaching critical values:

10.4.1 Code amber trigger value:

All interested parties, including the Adjoining Owner’s Surveyor and his Engineer, should be informed and further actions immediately agreed between two of the three Surveyors and implemented by the Building Owner. Notwithstanding the Party Wall requirements, the Contractor is to appoint a suitably qualified Structural Engineer who will be responsible for the reviewing of the movement monitoring results at the start and end of each day and provide immediate advice, remedial works and design as necessary in the event of movement being noted.

The Contractor is to ensure that he has 24 hour / 7 days a week access to emergency support provision including but not limited to additional temporary props, needles, waling beams and concrete supply at the start of the excavation and prior to any likelihood of this trigger value being reached. If this value is reached the Contractor, and his Engineer, provide all interested parties with his plan to implement any emergency remedial and supporting works deemed necessary. The Contractor must be ready to carry out these works without delay if the movement continues and approaches the trigger value below.

10.4.2 Code red trigger value:

All interested parties including Adjoining Owner’s Surveyor and Engineer will be informed immediately. Works will stop and be made safe using methods and equipment agreed at the above stage. The Contractor is to ensure that the movement has stopped as a result of the implemented remedial works designed and installed at this stage. The requirements of the Party Wall Act will also ensure that two of the three Surveyors and their advising Engineers shall then enter into an addendum Award, setting out whether or not the Building Owner’s works can re-commence and when, and if so agree on additional precautions or modifications to the proposals prior to recommencement.

Eleven

Ground movement assessment

11.1

A ground movement assessment will be completed by a specialist to take into account both the long and short-term effects of the proposed basement.

11.2

For a development of this size, we would expect that the analysis would conclude that the proposed basement excavations should not have an unacceptable impact on this property and the adjacent properties – the listed Braemar Avenue Baptist Church building and adjacent residential building. The neighbouring buildings are predicted to have Category 2 damage at worst.

The above damages are within the acceptable damage levels set out in the London Borough of Haringey subterranean development policies.

11.3

In order to mitigate the risk of Category 2 damage to the surrounding properties, the temporary works installed during the works will be designed to support the surcharge from the soil and surrounding buildings. A ground movement monitoring system will also be installed to the adjoining properties with trigger values set to allow the works to be controlled appropriately in the event of ground movement occurring.

With the implementation of these mitigation measures, any damage caused to the property and surrounding properties should be limited to Category 2 at worst.

Twelve

Conclusions

12.1

It is intended that the above measures and sequence of works are adopted for the eventual design and construction of the proposed works. If the works noted above are properly undertaken by suitably qualified contractors, these works should pose no significant threat to the structural stability of the house or the adjoining properties.

12.2

Detailed method statements and calculations for the enabling and temporary works will need to be prepared by the Contractor for comment by all relevant parties including party wall surveyors and their engineers. Elliott Wood will need to ensure that adequate supervision and monitoring are provided throughout the works particularly during the excavation and demolition stages.

12.3

A Burland Category report and Damage Risk Assessment will be prepared by site investigation company which is included in the site investigation.

12.4

To this end, Elliott Wood will have an on-going role during the works on site to monitor that the works are being carried out generally in accordance with our design and specification. This role will typically involve weekly site visits at the beginning of the project and fortnightly thereafter. A written site report is provided to the design team, Contractor and Party Wall Surveyor.

Thirteen

Subterranean construction method statement

13.1 Construction generally

It is assumed that the above measures and assumed sequence of works are taken into account in the eventual design and construction of the proposed works.

Detailed method statements and calculations for the enabling and temporary works will need to be prepared by the Contractor for comment by all relevant parties including Party Wall Surveyors and their Engineers. Elliott Wood will need to ensure that adequate supervision and monitoring is provided throughout the works particularly during the excavation and demolition stages.

To this end, Elliott Wood will have an on-going role during the works on site to monitor that the works are being carried out generally in accordance with our design and specification. This role will typically involve weekly site visits at the beginning of the project and fortnightly thereafter. A written site report is provided to the design team, Contractor and Party Wall Surveyor.

Access onto the site will be from Campden Hill Square and must be coordinated in a sensible manner to minimise disruption to the adjoining residents and provide a safe working environment.

Stage 1: Site set-up

- Erect a fully enclosed painted plywood site hoarding along the front boundary wall, this should not impede on the neighbouring properties.
- The services within the site should be identified and isolated as necessary. All below ground obstructions should also be removed to allow the works to progress.
- The principles for the removal of spoil shall be agreed. Given the scope of the works, it is likely that conveyors will be used to move the spoil from within the building to a holding skip located in the front garden/driveway. Grab lorries will be used to remove the material from the skip. Refer to the CTMP for detailed information on the site set-up and waste removal.
- Tree protection methods to be agreed and installed to all retained trees.
- Monitoring points should be installed to all neighbouring structures and infrastructure and a base reading should be taken prior to any construction works starting on the site.

Stage 2: Internal soft strip & demolition

- The complete soft strip of internal finishes within the building.
- Carefully demolish the existing buildings down to ground floor level in a staged sequence (TBC by the Contractor).

Stage 3: Install piles

- Install secant or contiguously piled retaining walls along the perimeter of the basement excavation as per the architectural and structural drawings.
- The type of piled wall to be installed will be confirmed upon receipt of the geotechnical site investigation report and will be agreed with the geotechnical engineer in charge.

Stage 4: Bulk excavation

- Reduce level dig down to approx. 1m below ground level.
- Install steel waling beams around the perimeter of the excavation.
- Install horizontal props spanning across the width of the basement between the waling beams.
- Continue excavating down to formation level in stages, the level at which propping is required is to be determined by the temporary works engineer. The propping levels will take into account the permanent works design such that the RC slabs can be cast above/below the props whilst the props remain in place.
- Install RC liner walls and base slab over blinding once formation level is reached.

Stage 5: Cast RC base slab

- At formation level cast blinding layer at central section and install the below ground drainage as required.
- Install compressible void former under areas of suspended slab if required – TBC by geotechnical engineer.
- Install and tie reinforcement for RC liner wall base slabs. Central section of basement slab will be connected to the basement wall bases with steel dowels.
- Cast the RC basement slab.
- Once the base slab has cured it will provide a permanent low-level prop to the basement retaining walls/piles and hence, the lowest level of horizontal propping and waling beams can be removed if installed – TBC by temporary works engineer.

Stage 6: Cast RC top slab (at ground floor level)

- The remainder of the internal basement walls and columns can be constructed up to ground floor level. The temporary props should be retained and re-propped off the new walls.
- Once the walls and columns have cured cast the ground floor slab. Once the ground floor slab has cured it will provide a permanent high-level prop to the basement retaining walls and hence, the remaining horizontal propping and waling beams can be removed.

Stage 7: Construct superstructure

- Once the ground floor slab has cured, the superstructure works can commence.

Fourteen

Dust, noise and vibration

14.1

The design team and the contractor is to ensure that any construction impacts such as noise, vibration and dust are kept to acceptable levels for the duration of the works.

Those most likely to be affected by noise dust and vibration will be the immediate neighbours. There may be some impact on other residents on the street due to the related construction traffic, but this should be minimal.

Below we have described the mitigation measures that are proposed to keep noise, dust and vibration to acceptable levels.

14.2 Mitigation measures for demolition of existing building

The breaking out of existing structures shall be carried out by diamond saw cutting and hydraulic bursting where possible to minimise noise and vibration to the adjacent properties. All demolition and excavation work will be undertaken in a carefully controlled sequence, taking into account the requirement to minimise vibration and noise. The contractor will need to utilise non-percussive breaking techniques where practicable.

Dust suppression equipment should be used during the demolition process to ensure that any airborne dust is kept to a minimum. Where practical, concrete should also be wetted down prior to and during breakout to further inhibit airborne dust.

14.3 Mitigation measures for bulk excavation

Due to the size of the basement, it is likely that some mechanical plant will be required to complete the bulk excavation. The contractor should ensure that any mechanical plant is switched off when not in use and is subject to regular maintenance checks and servicing. An electrically powered conveyor will be used as detailed above.

14.4 Mitigation measures for the construction of the concrete basement shell

The contractor should ensure that any concrete pours are completed within the permitted hours for noise generating works. The contractor should allow for a contingency period to ensure that concrete pours can be completed within these hours regardless of unforeseen circumstances such as batching plant delays and traffic congestion.

The fabrication and cutting of steelwork for the reinforced concrete elements shall take place off site. If any rebar needs to be trimmed on site this should be completed using hydraulic or pneumatic tools instead of angle grinders.

14.5 Mitigation measures for piling

The piles will be formed using a continuous flight auger rig – this is a non-percussive technique and therefore produces significantly less noise and vibration than the alternative driven piles. Some of the temporary piles will require breaking down to slab level once the basement works are complete. The contractor should ensure that they use non-percussive pile reduction techniques which are much quieter than traditional breakers.

14.6 Dust control

In order to reduce the amount of dust generated from the site, the contractor should ensure that any cutting, grinding and sawing should be completed off site where practicable. If cutting, grinding and sawing is being carried out on site, surfaces are to be wetted down prior to and during these types of work whenever possible. Any equipment used on site should be fitted with dust suppression or a dust collection facility.

The contractor will be responsible for ensuring good practice with regards to dust and should adopt regular sweeping, cleaning and washing down of the hoardings and scaffolding to ensure that the site is kept within good order. The Contractor selected will be a member of the Considerate Contractors Scheme. Contact details of the contractor who will be responsible for containing dust and emissions within the site will be displayed on the site boundary so that the local residents can contact the contractor to raise any concerns regarding noise and dust.

The building will be enclosed within suitable scaffold sheeting and any stockpiles of sand or dust-generating materials will be covered. Cement, fine aggregates, sand and other fine powders should be sealed after use.



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Appendices

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A Proposed structural layouts

KEY

C1

- 250x350mm RC columns

W

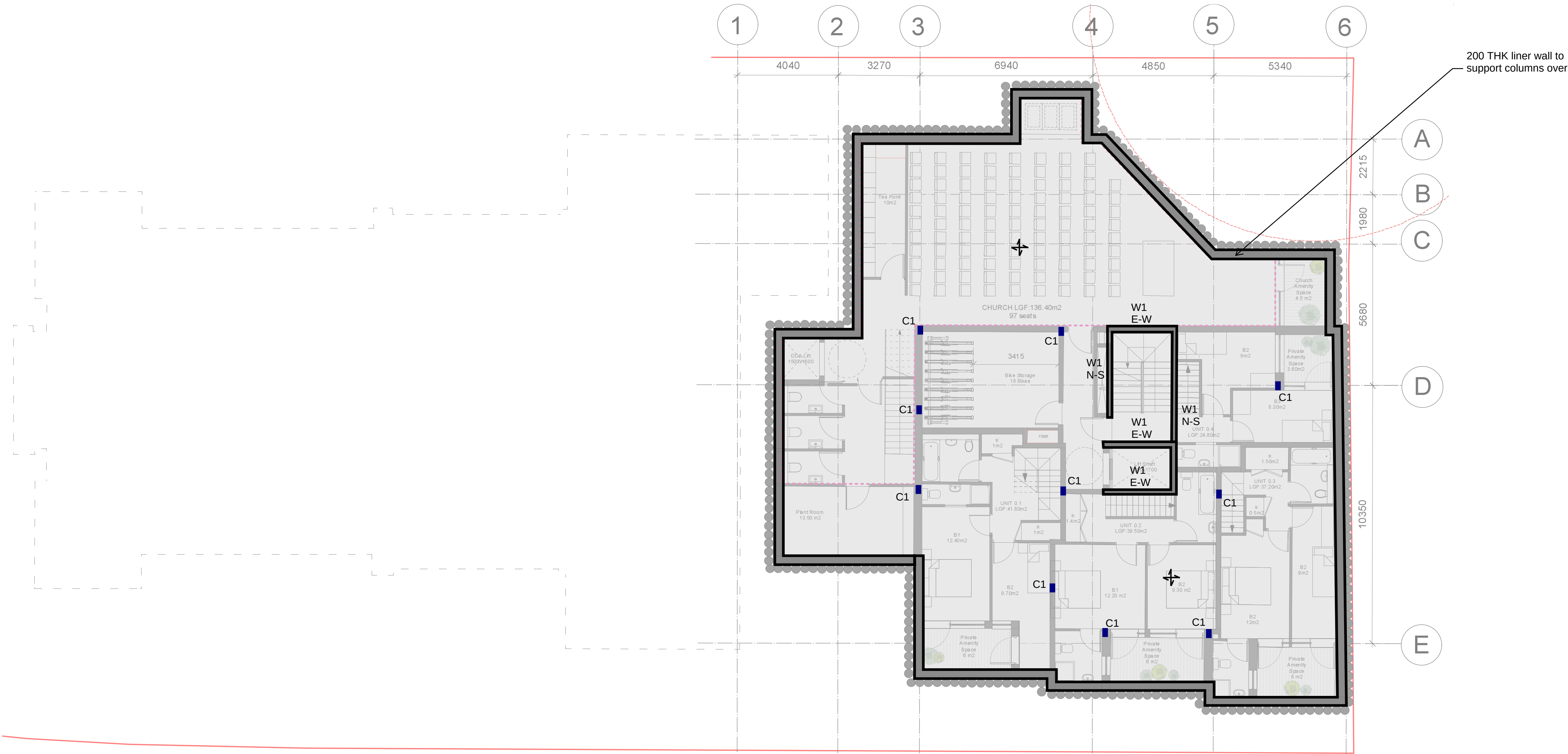
1

- 200mm thick RC walls

+

1

- 400mm dp RC raft slab



This drawing is to be read in conjunction with all relevant architects, engineers and specialists drawings and specifications.

Do not scale from this drawing.

- Notes**
- Indicative sketches for preliminary advice only.
 - Any changes to the architectural scheme will necessitate rechecking the structural scheme

P1	18/11/22	TU	JCa	Preliminary loads
rev	date	by	chk	description

scale (s) NTS date November 2022 drawn TU

Drawing title
Proposed basement layout
Option 1

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Project
Braemar Avenue, Wood Green

Drawing status	Status	Revision
Preliminary	S2	P1

Project no.	Originator	Zone	Level	Type	Role	drg no.
2220367-EWP-ZZ-B1-SK	-	-	-	-	-	S-0001

KEY

c1

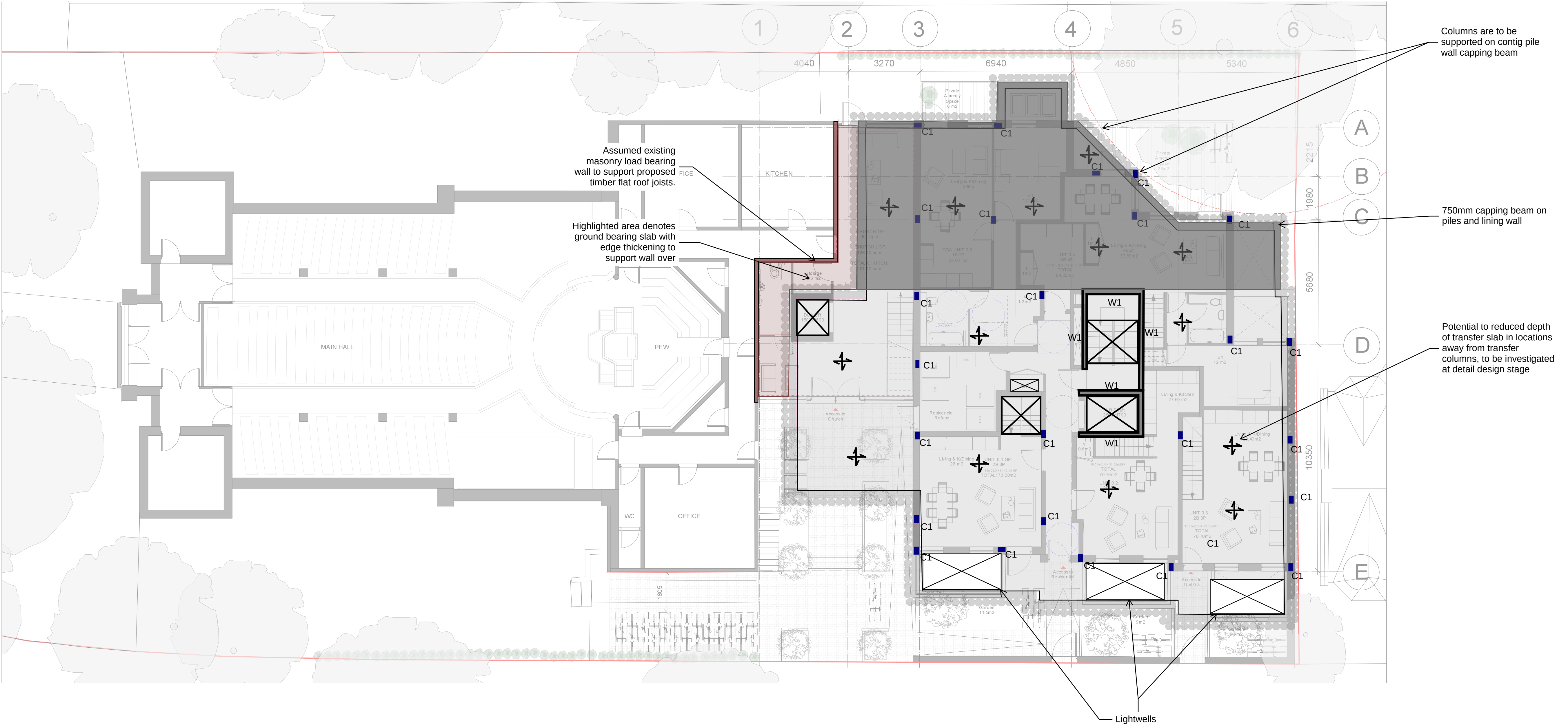
- 250x350mm RC columns

w1

- 200mm thick RC walls

- 450mm dp transfer RC slab

- 225mm dp RC slab



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Do not scale from this drawing.

- Notes**
- Indicative sketches for preliminary advice only.
 - Any changes to the architectural scheme will necessitate rechecking the structural scheme.
 - Pile Diameters TBC
 - Foundation solution in abeyance, pending ground investigation results

P1	23/11/22	TU	JCa	Preliminary
rev	date	by	chk	description

Drawing title

Proposed GF layout - Option 1

scale (s)	date	drawn
NTS	November 2022	TU

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Project

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Drawing status

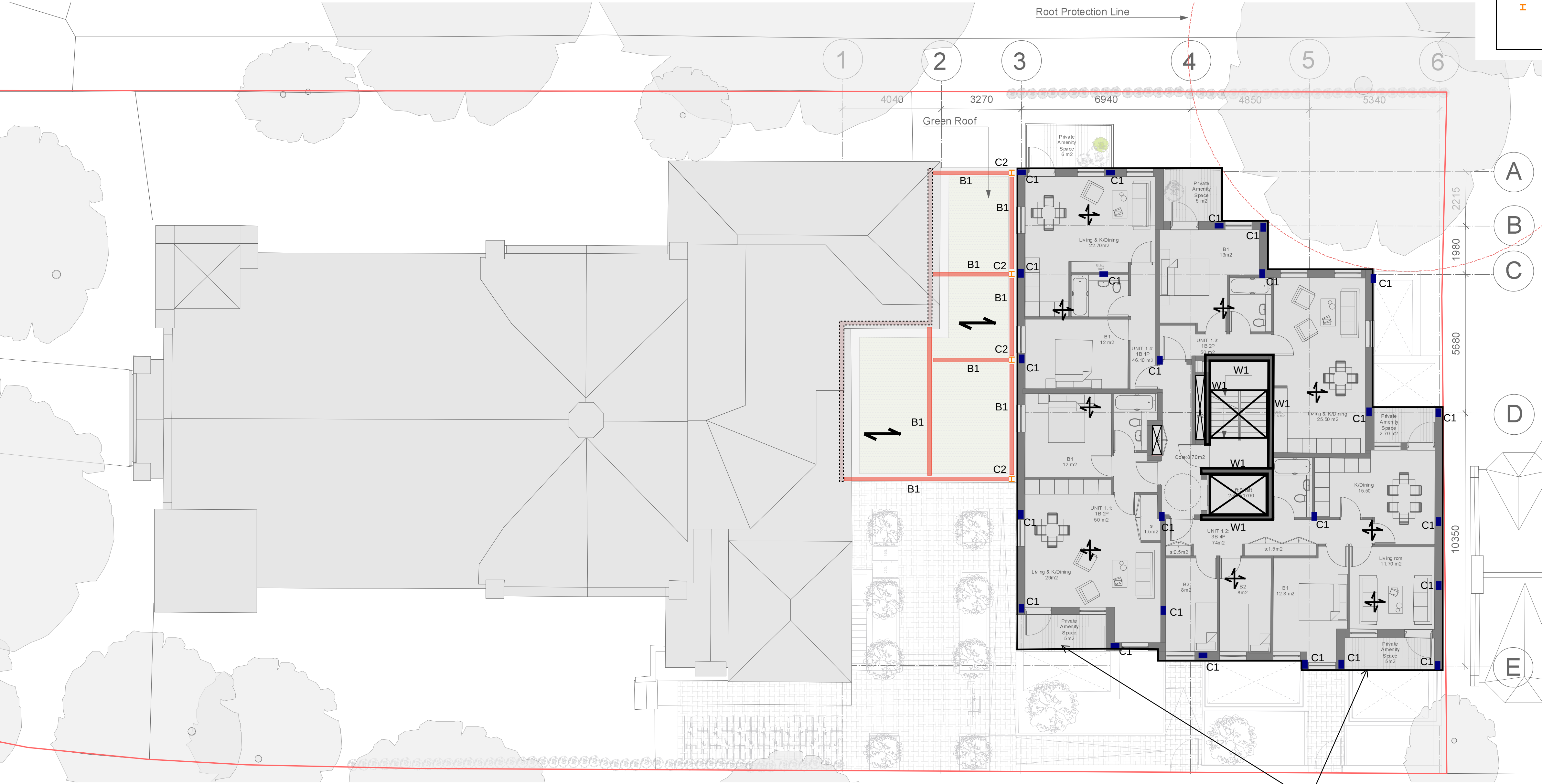
Preliminary

Status Revision

S2 P1

Project no.

2220367-EWP-ZZ-00-SK-S-0002



KEY

C1

- 250x350mm RC columns

W1

- 200mm thick RC walls

+

+

- 225mm dp RC slab

B1

- 254UB steel roof beam

↗

↘

- 50x250 deep C24 timber joists @ 200mm c/c supporting 2 layers of 12mm ply glued and screwed to joists

↑

↑

- 203UC steel posts to support 1 storey building

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P1	23/11/22	TU	JCa	Preliminary
rev	date	by	chk	description

Drawing title

Proposed first floor layout - Option 1

scale (s)	date	drawn
NTS	November 2022	TU

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Drawing status	Status	Revision
Preliminary	S2	P1

Project no.	Originator	Zone	Level	Type	Role	Role	drg no.
2220367-EWP-ZZ-01-SK	S	S	S	S	S	S	0003

KEY

c1

- 250x350mm RC columns

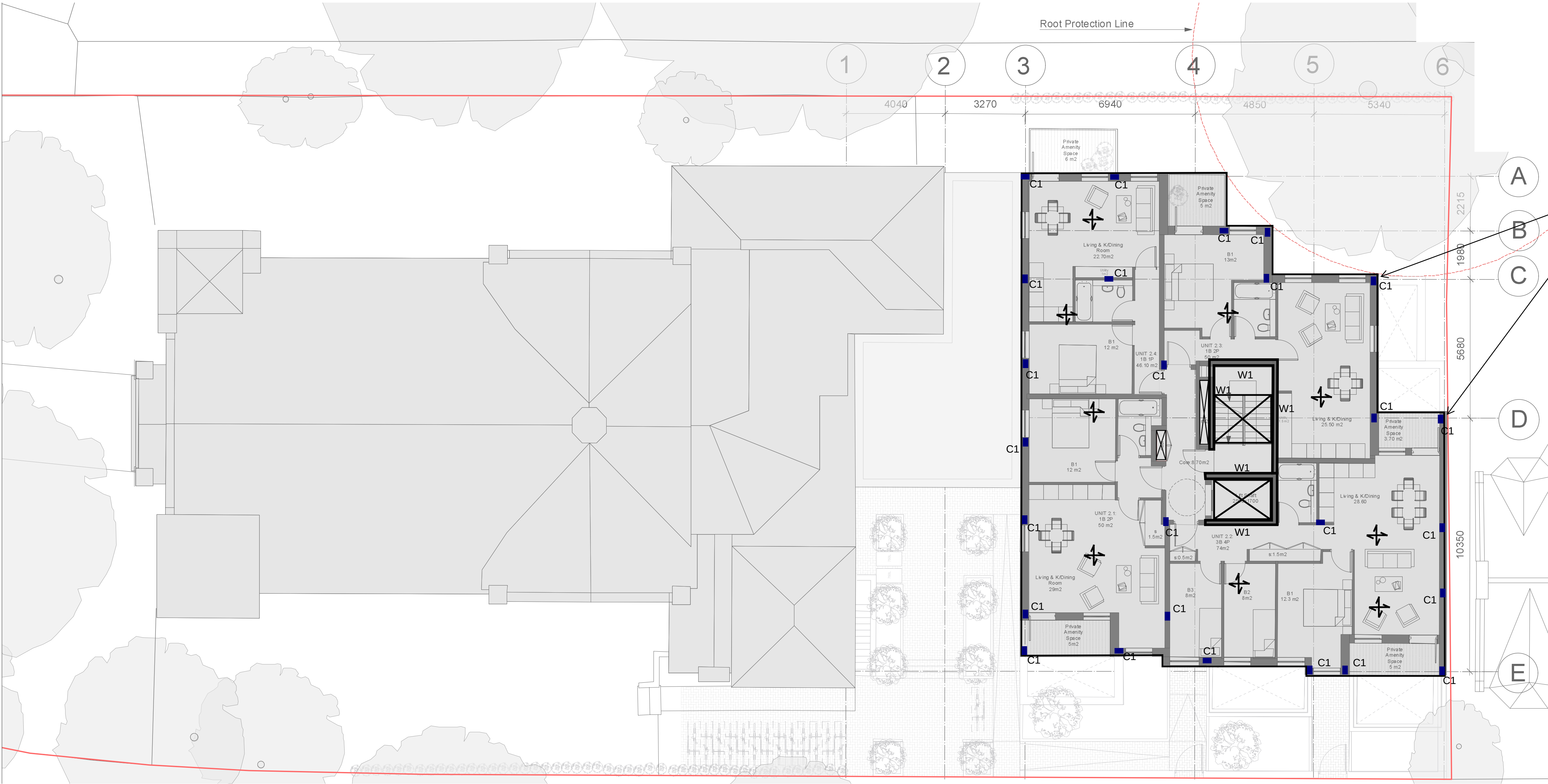
w1

- 200mm thick RC walls

+

+

- 225mm dp RC slab



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Do not scale from this drawing.

- Notes**
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 - Any changes to the architectural scheme will necessitate rechecking the structural scheme.

P1	23/11/22	TU	JCa	Preliminary
rev	date	by	chk	description

Drawing title

Proposed second floor layout - Option 1

scale (s)	date	drawn
NTS	November 2022	TU

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Drawing status	Status	Revision
Preliminary	S2	P1

Project no.	Originator	Zone	Level	Type	Role	drg no.
2220367-EWP-ZZ-02-SK	-	-	-	-	-	S-0004

KEY

c1

- 250x350mm RC columns

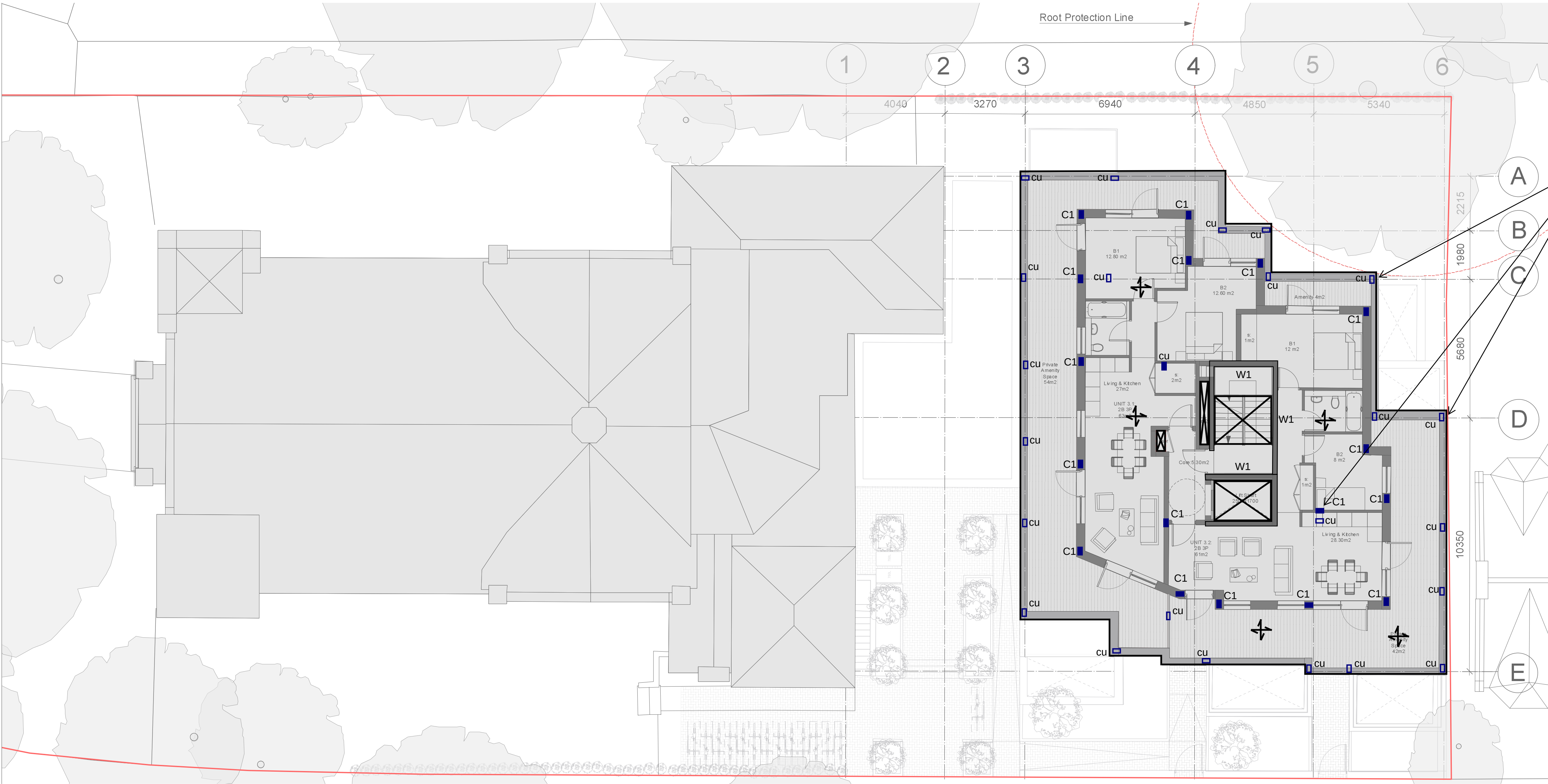
cu

- Column Under

w1

- 200mm thick RC walls

- 300mm dp RC transfer slab



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- Notes**
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 - Any changes to the architectural scheme will necessitate rechecking the structural scheme.

P1	23/11/22	TU	JCa	Preliminary
rev	date	by	chk	description

Drawing title

Proposed third floor layout - Option 1

scale (s)	date	drawn
NTS	November 2022	TU

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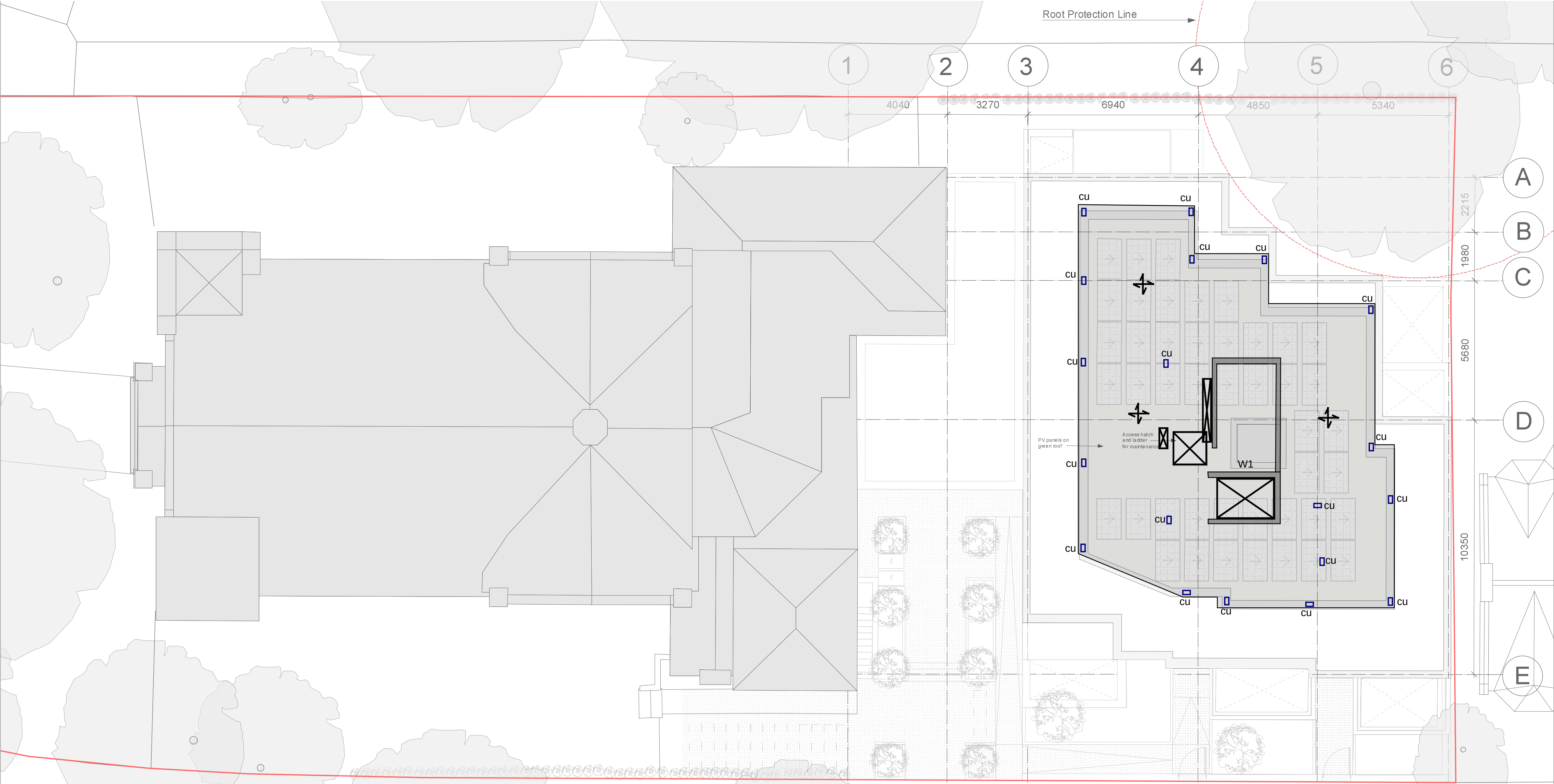
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Project

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Drawing status	Status Revision
Preliminary	S2 P1

Project no.	Originator	Zone	Level	Type	Role	drg no.
2220367-EWP-ZZ-03-SK	-	-	-	-	-	-S-0005



KEY

c1

- 250x350mm RC columns

cu

- Column Under

w1

- 200mm thick RC walls

- 150mm dp RC roof slab

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- Notes**
- Indicative sketches for preliminary advice only.
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P1	23/11/22	TU	JCa	Preliminary
rev	date	by	chk	description

Drawing title

Proposed roof layout - Option 1

scale (s)	date	drawn
NTS	November 2022	TU

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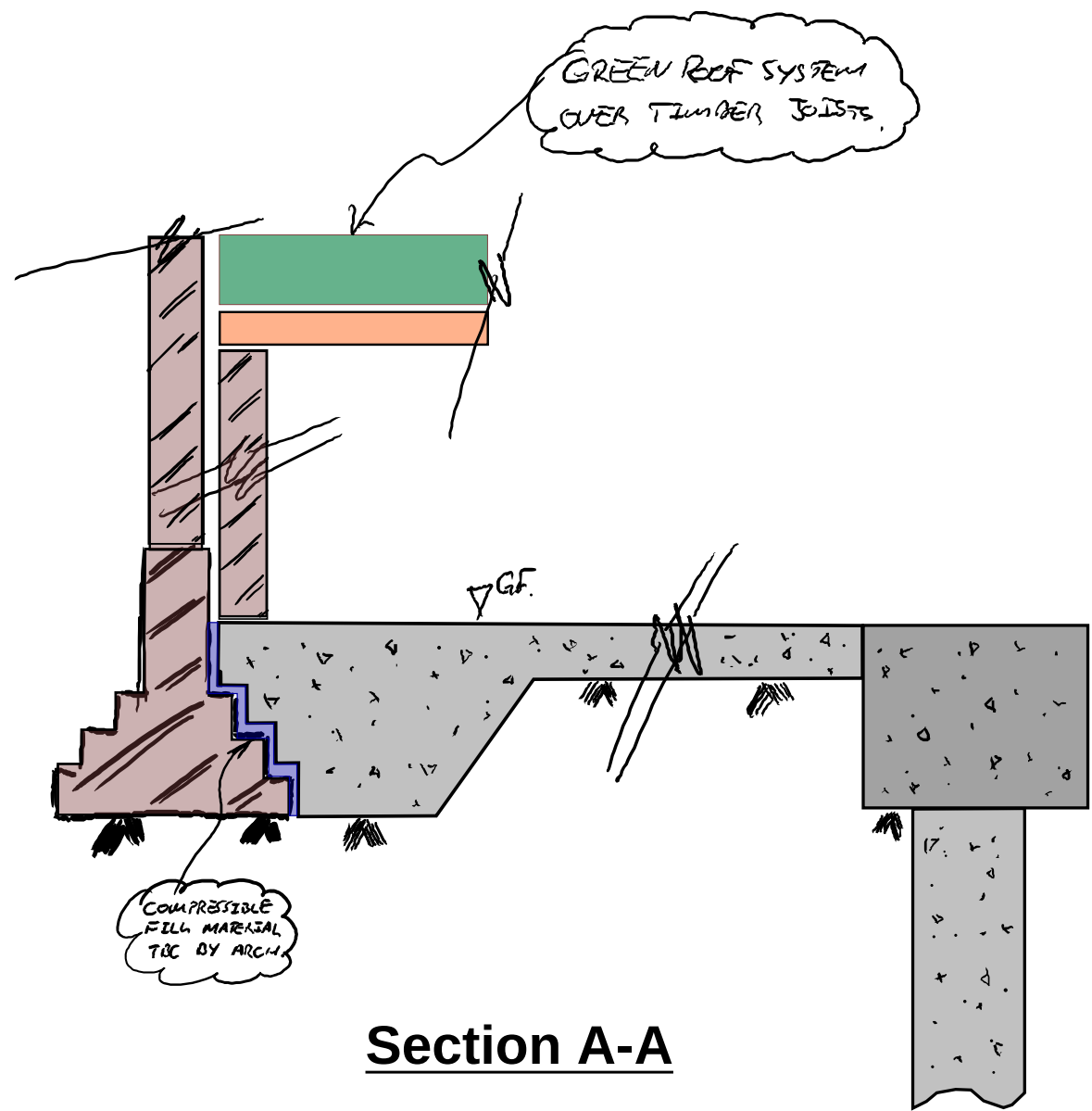
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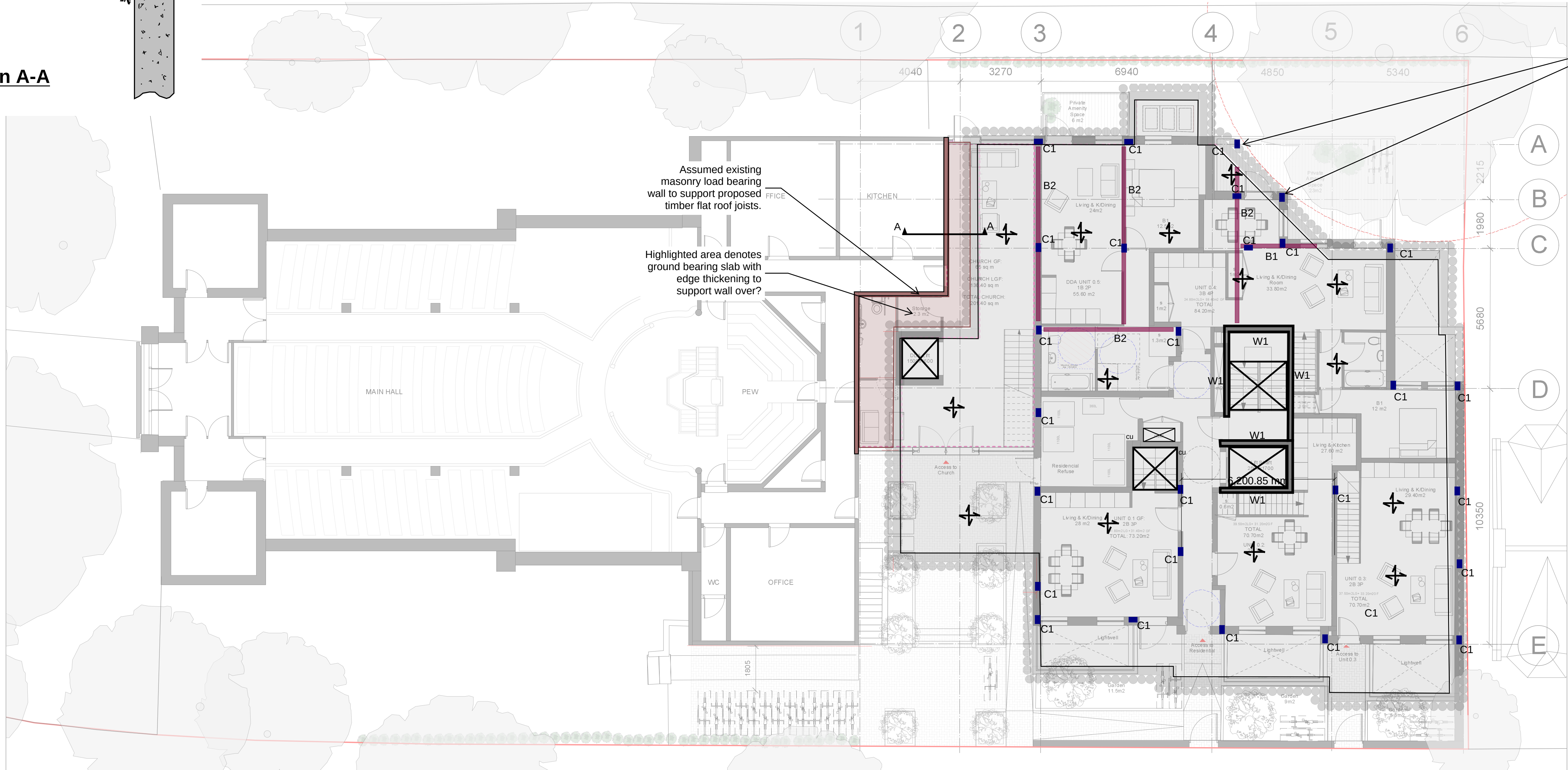
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Project		Braemar Avenue, Wood Green					
Drawing status		Preliminary		Status		Revision	
				S2		P1	
Project no.	Originator	Zone	Level	Type	Role	drg no.	
2220367	EW	P	RF	SK	S	0006	



KEY	
	- 250x350mm RC columns
	- 200mm thick RC walls
	- 225mm dp RC slab
	- 250x500mm dp RC beam
	- 300x650mm dp RC beam



Columns are to be supported on contig pile wall capping beam

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Notes
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P1	23/11/22	TU	JCa	Preliminary
rev	date	by	chk	description

Drawing title
Proposed GF layout - Option 2

scale (s)	date	drawn
NTS	November 2022	TU

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Drawing status		Status Revision	
Preliminary		S2	P1
Project no.	Originator Zone	Level	Type Role
2220367-EWP-ZZ-00-SK			S-0102

KEY

c1

- 250x350mm RC columns

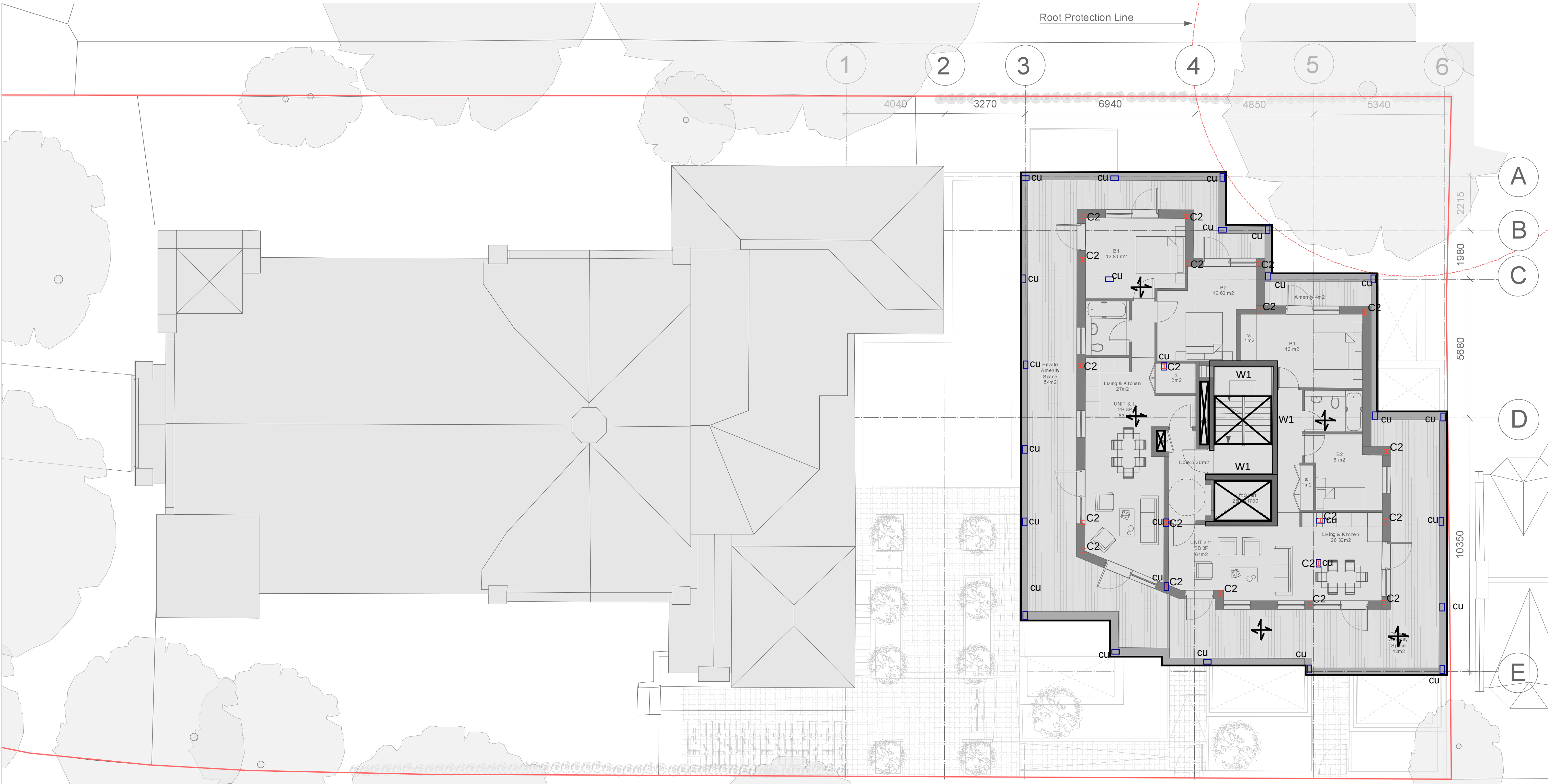
w1

- 200mm thick RC walls

- 275mm dp RC transfer slab

c2

- 152UC steel column



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Do not scale from this drawing.

- Notes**
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P1	23/11/22	TU	JCa	Preliminary
rev	date	by	chk	description

Drawing title

Proposed third floor layout - Option 2

scale (s)	date	drawn
NTS	November 2022	TU

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Project

Braemar Avenue, Wood Green

Drawing status

Preliminary

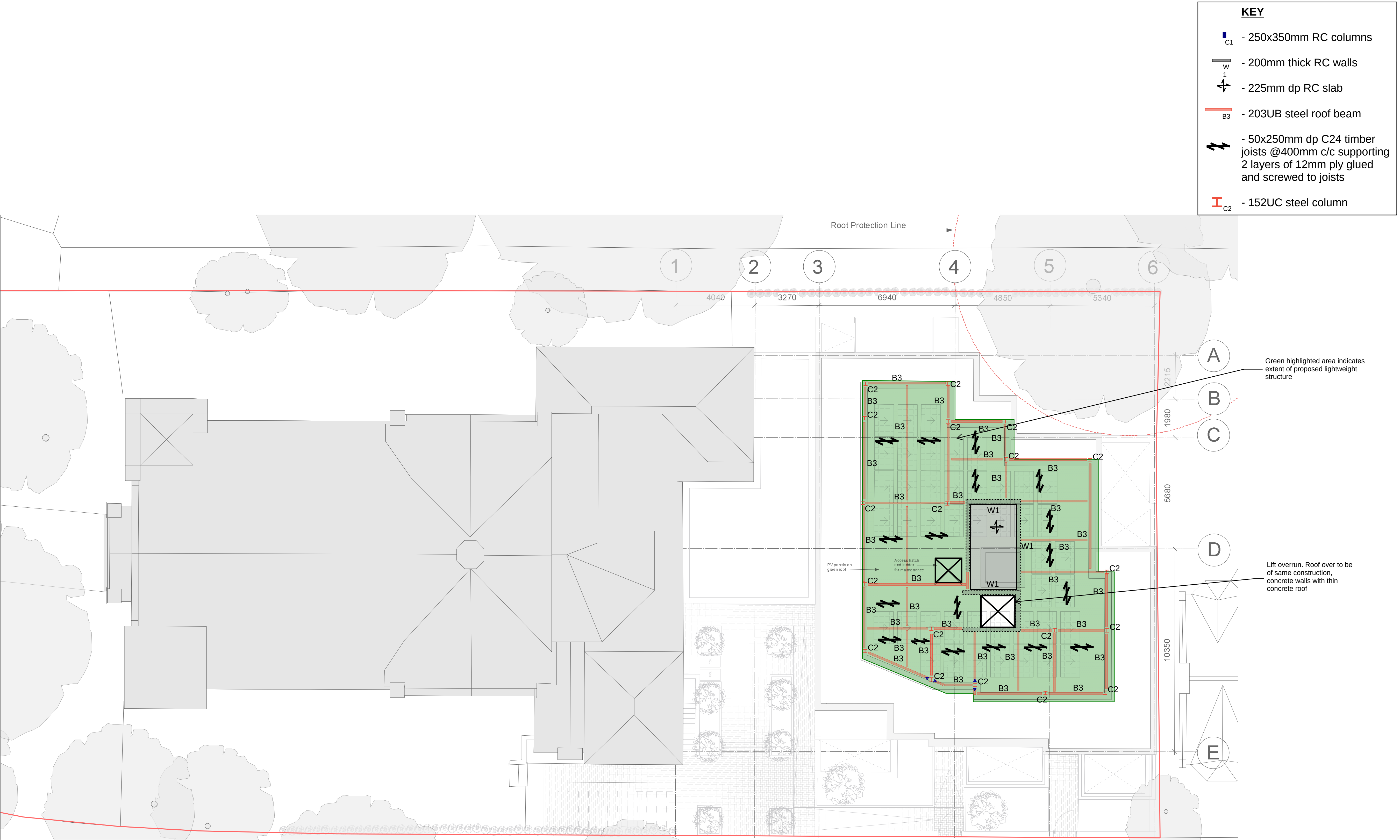
Status Revision

S2 P1

Project no.

2220367-EWP-ZZ-03-SK-S-0105

Originator Zone Level Type Role drg no.



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Notes

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P1	23/11/22	TU	JCa	Preliminary
rev	date	by	chk	description

Drawing title

Proposed roof layout - Option 2

scale (s)	date	drawn
NTS	November 2022	TU

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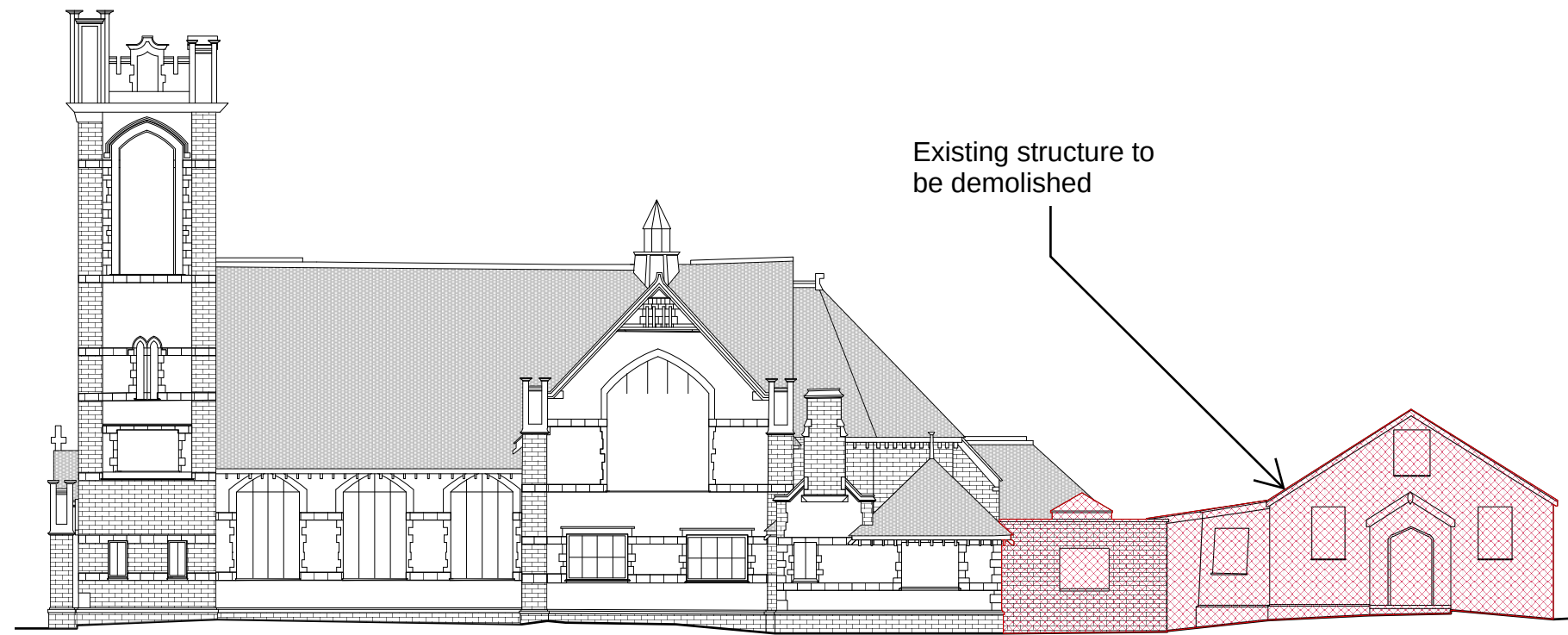
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Project

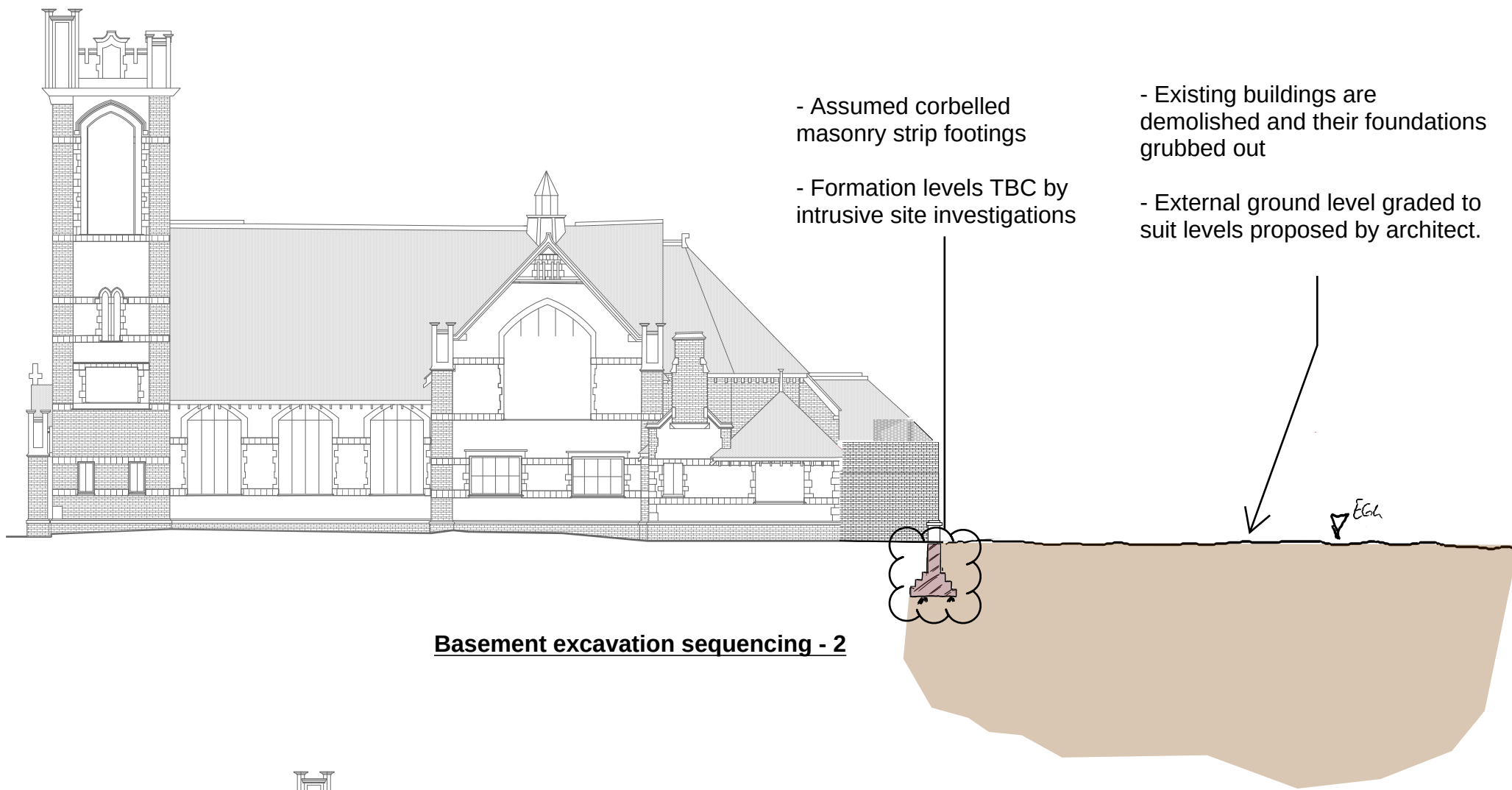
Braemar Avenue, Wood Green

Drawing status		Status Revision			
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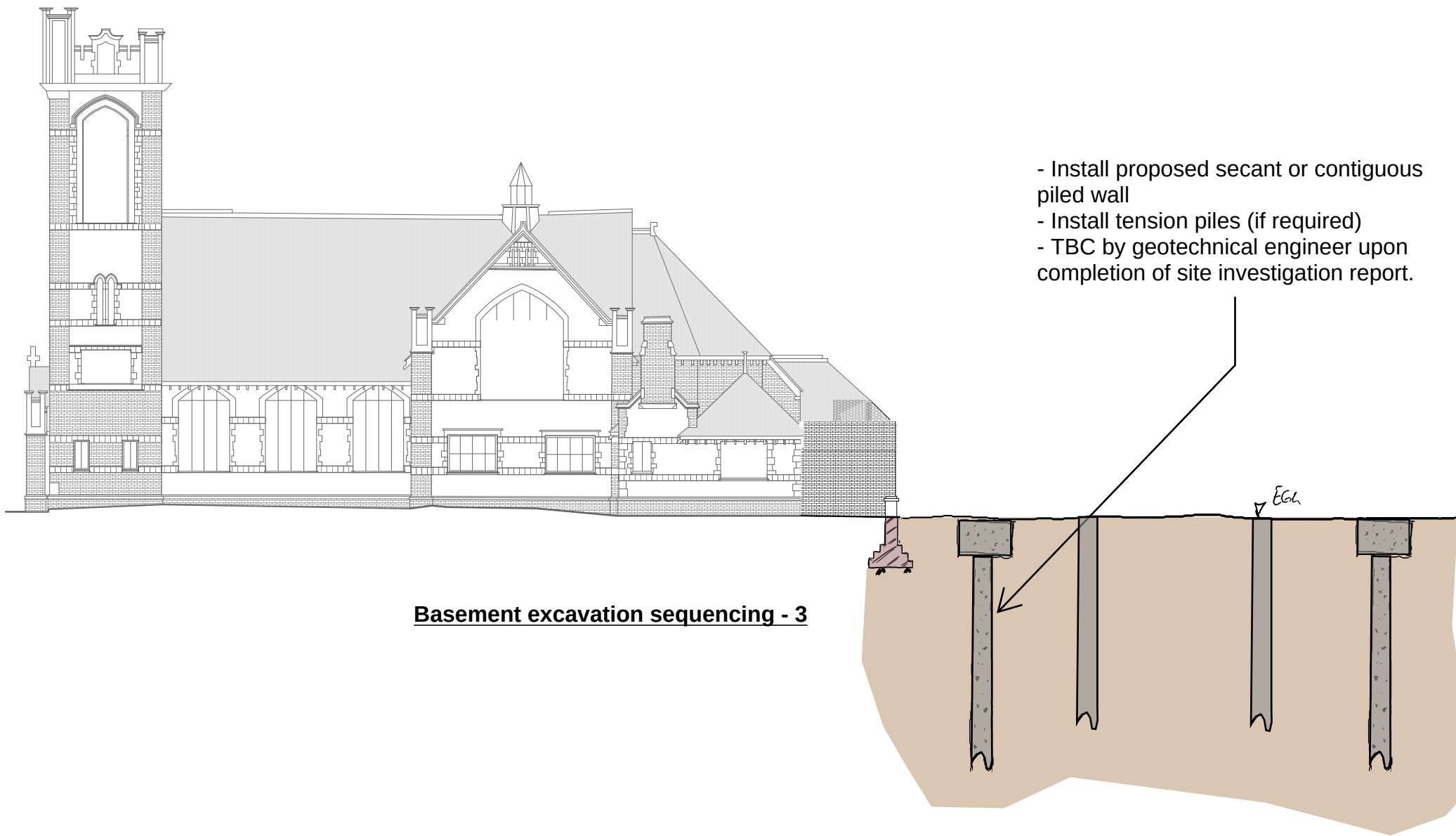
B Proposed Basement Construction Sequence



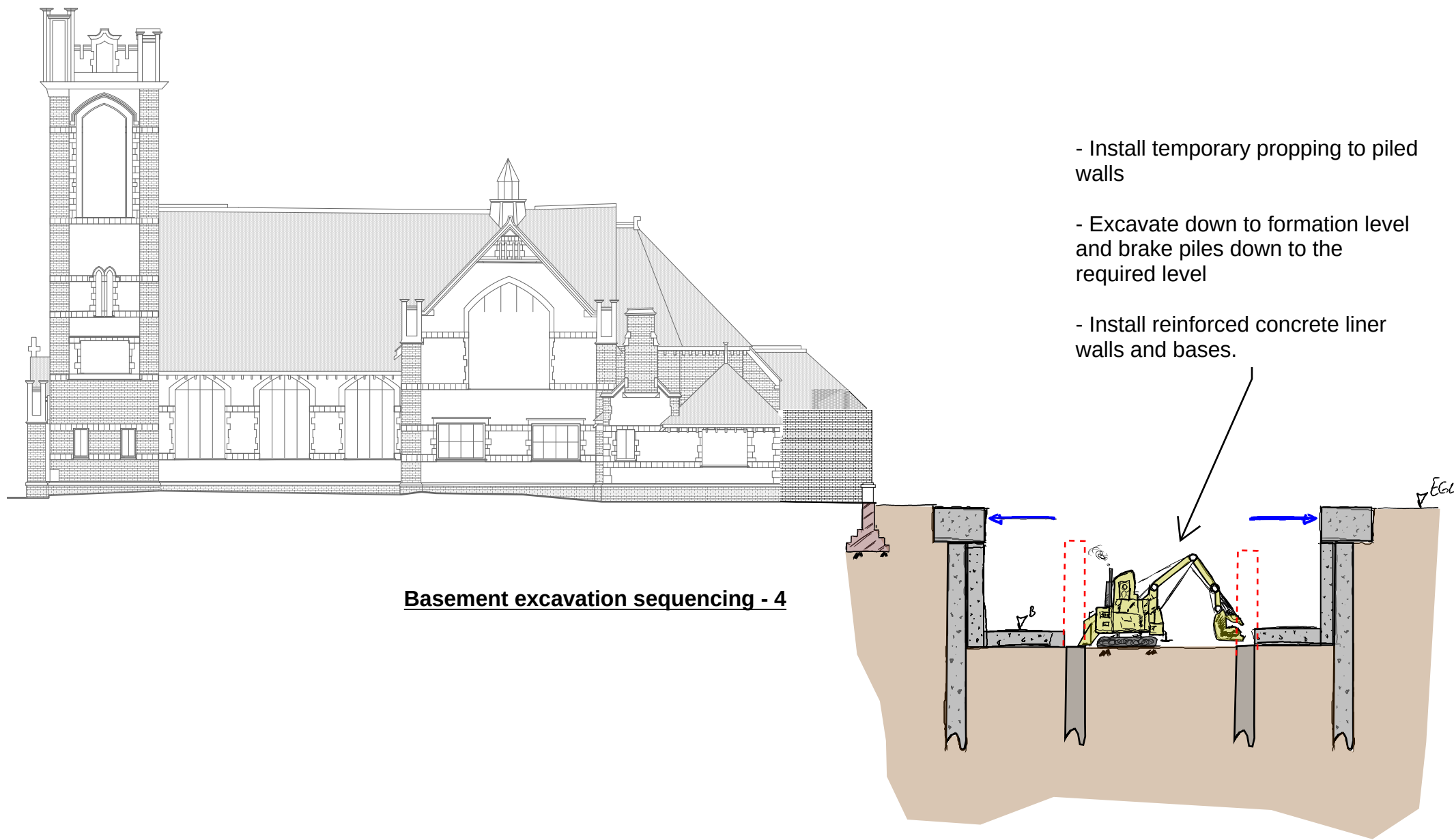
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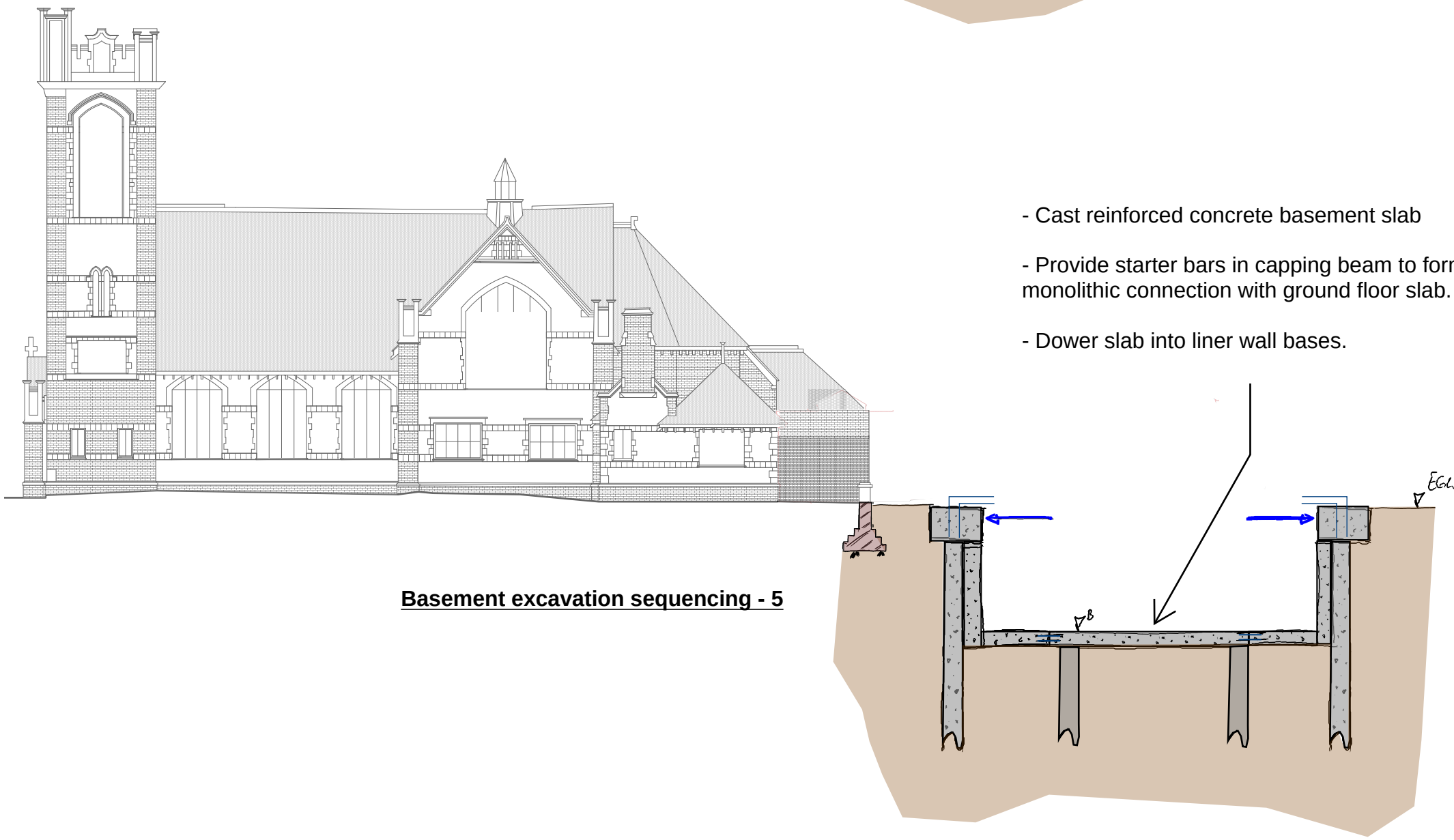
Basement excavation sequencing - 2



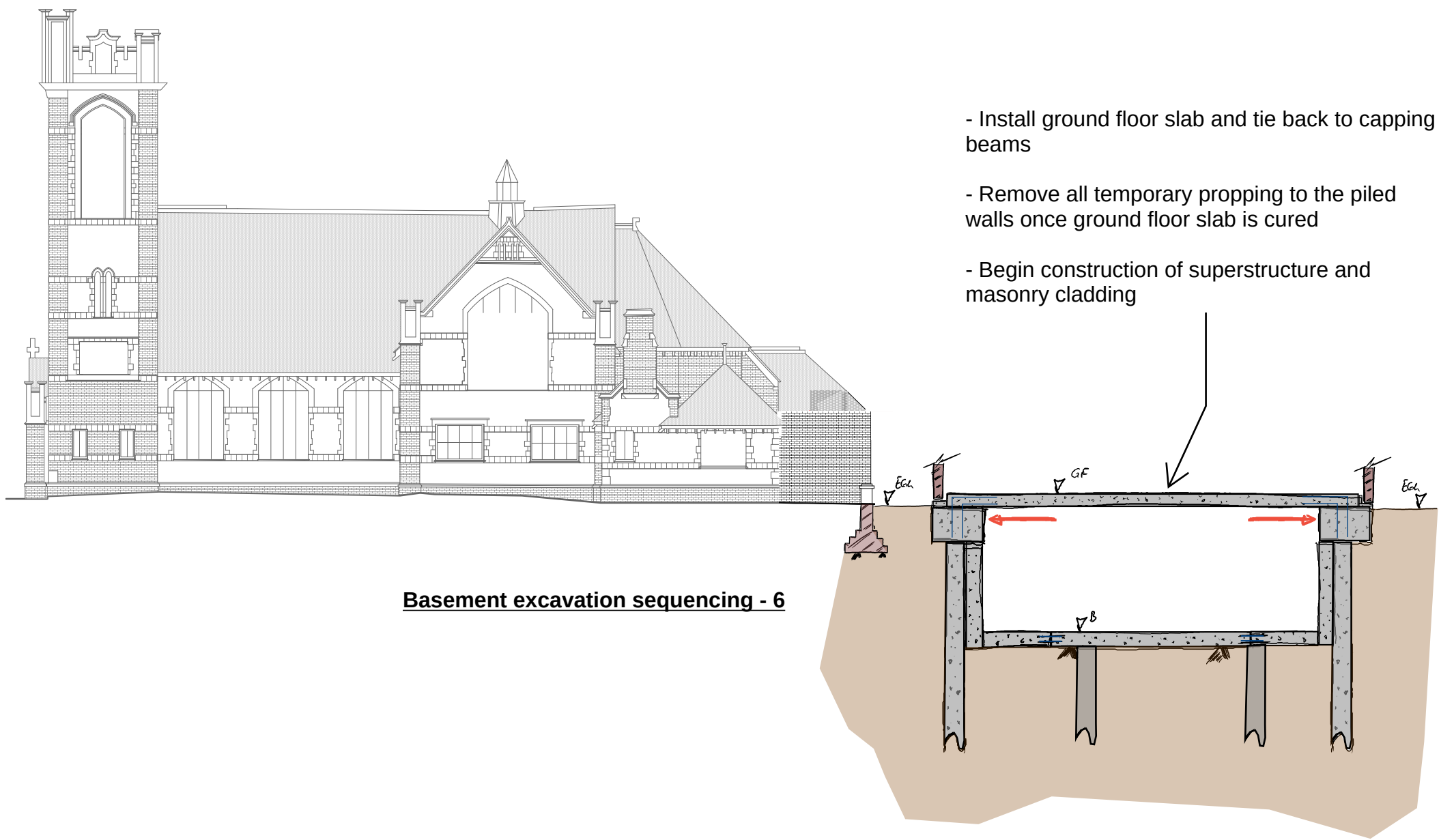
Basement excavation sequencing - 3



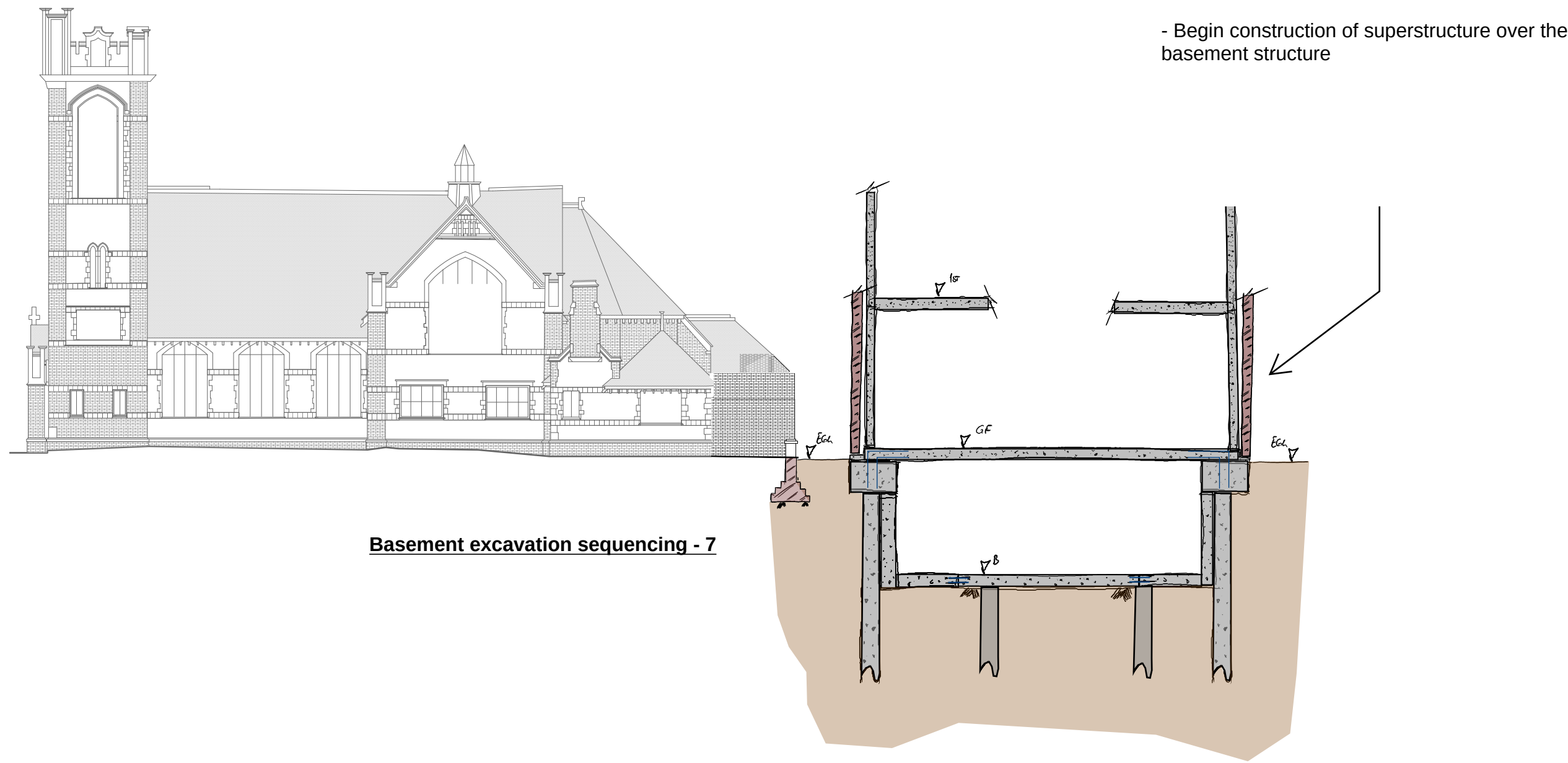
Basement excavation sequencing - 4



Basement excavation sequencing - 5



Basement excavation sequencing - 6



Basement excavation sequencing - 7

This drawing is to be read in conjunction with all relevant architects, engineers and specialists drawings and specifications.

Do not scale from this drawing.

NOTES

- This sketch is to be read in conjunction with EWP report no. 2220367-EWP-ZZ-XX-RP-S-0001_P01 - Structural Engineering Report & Subterranean Construction Method Statement

- This sketch is indicative only and not for tender or construction.

- Foundation type and details TBC upon receipt of geotechnical site investigation report.

P1	22/11/22	TU	JC	Preliminary sequencing
rev	date	by	chk	description

Drawing title
Preliminary and indicative
basement excavation sequence

scale (s)	date	drawn
NTS	November 2022	TU

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Project
Braemar Avenue, Wood Green

Drawing status
Preliminary

Status Revision
S2 P1

Project no. Originator Zone Level Type Role drg no.
2220367-EWP-ZZ-XX-SK-S-0008



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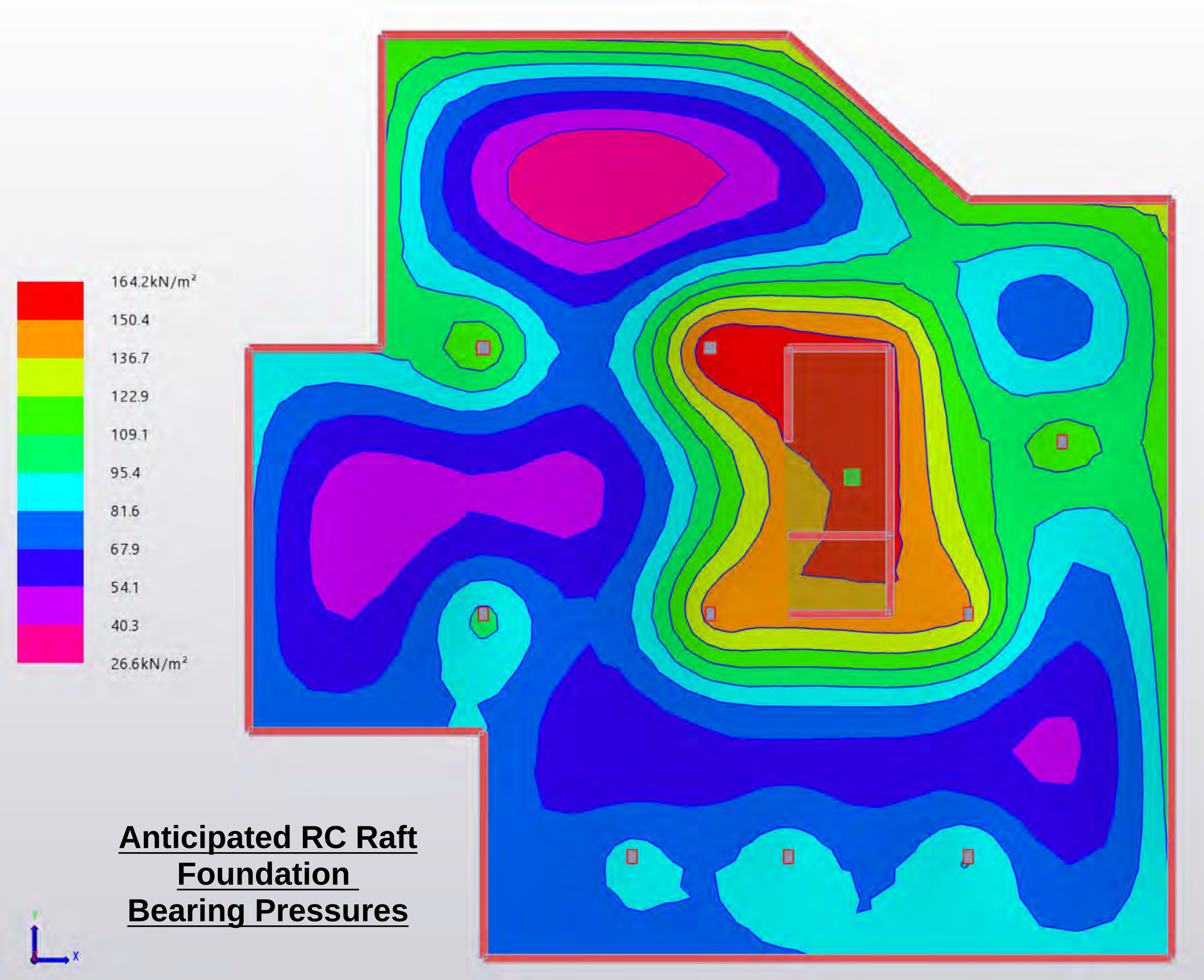
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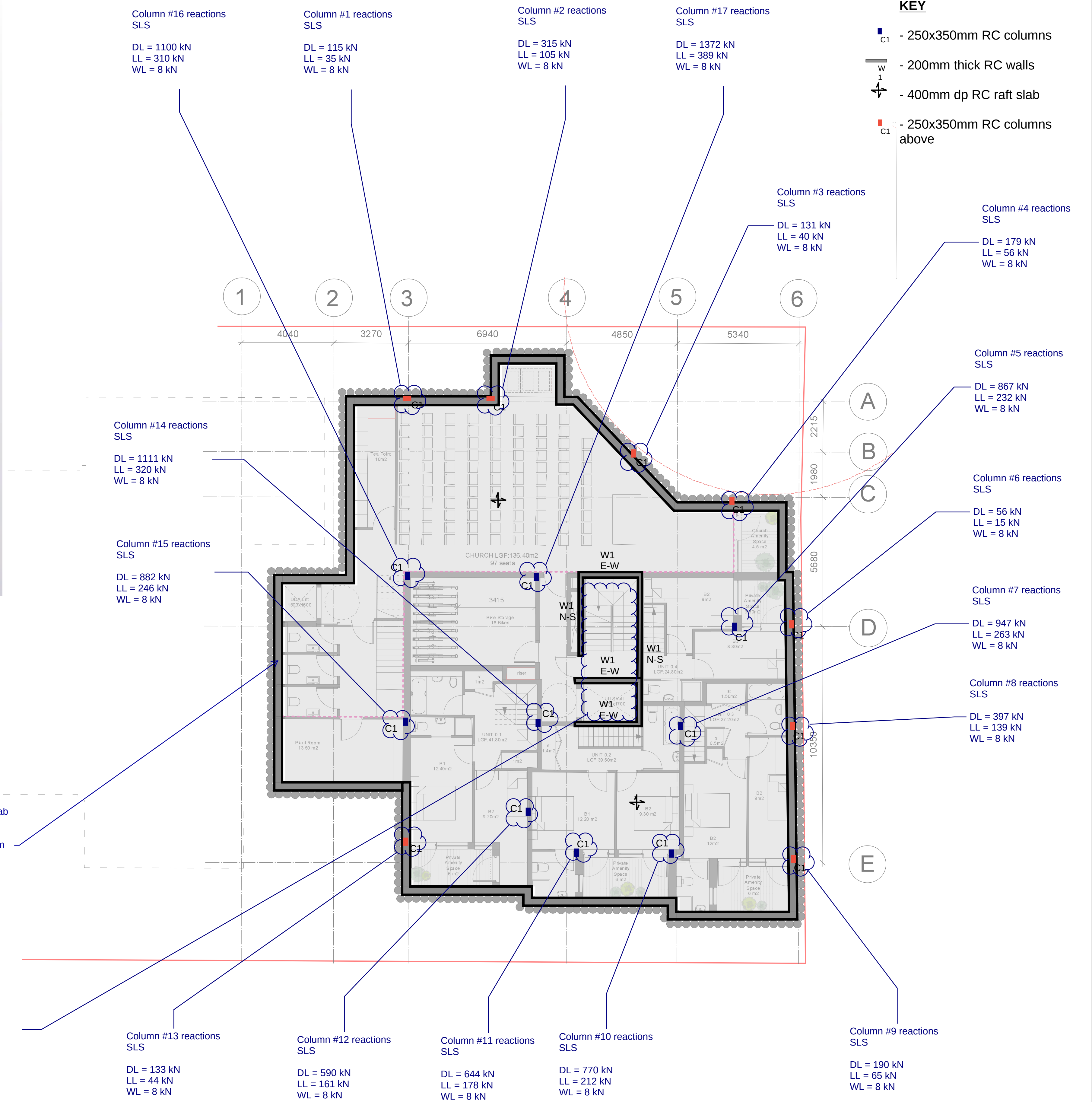
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Anticipated RC Raft
Foundation
Bearing Pressures



This drawing is to be read in conjunction with all relevant architects, engineers and specialists drawings and specifications.
Do not scale from this drawing.

Notes
- Indicative sketches for preliminary advice only.
- Any changes to the architectural scheme will necessitate rechecking the structural scheme to ensure that superstructure loads are reflected accordingly.
- All loads are split into dead, live, wind and are given as unfactored.
- Foundations are assumed to be a 400mm dp RC raft foundation - TBC by geotechnical engineer.

- The above column loads do not include the proposed raft foundation
- Allow for a 16 kPa (DL+LL) area load to account for self-weight, finishes and imposed loading on basement slab / raft foundation.

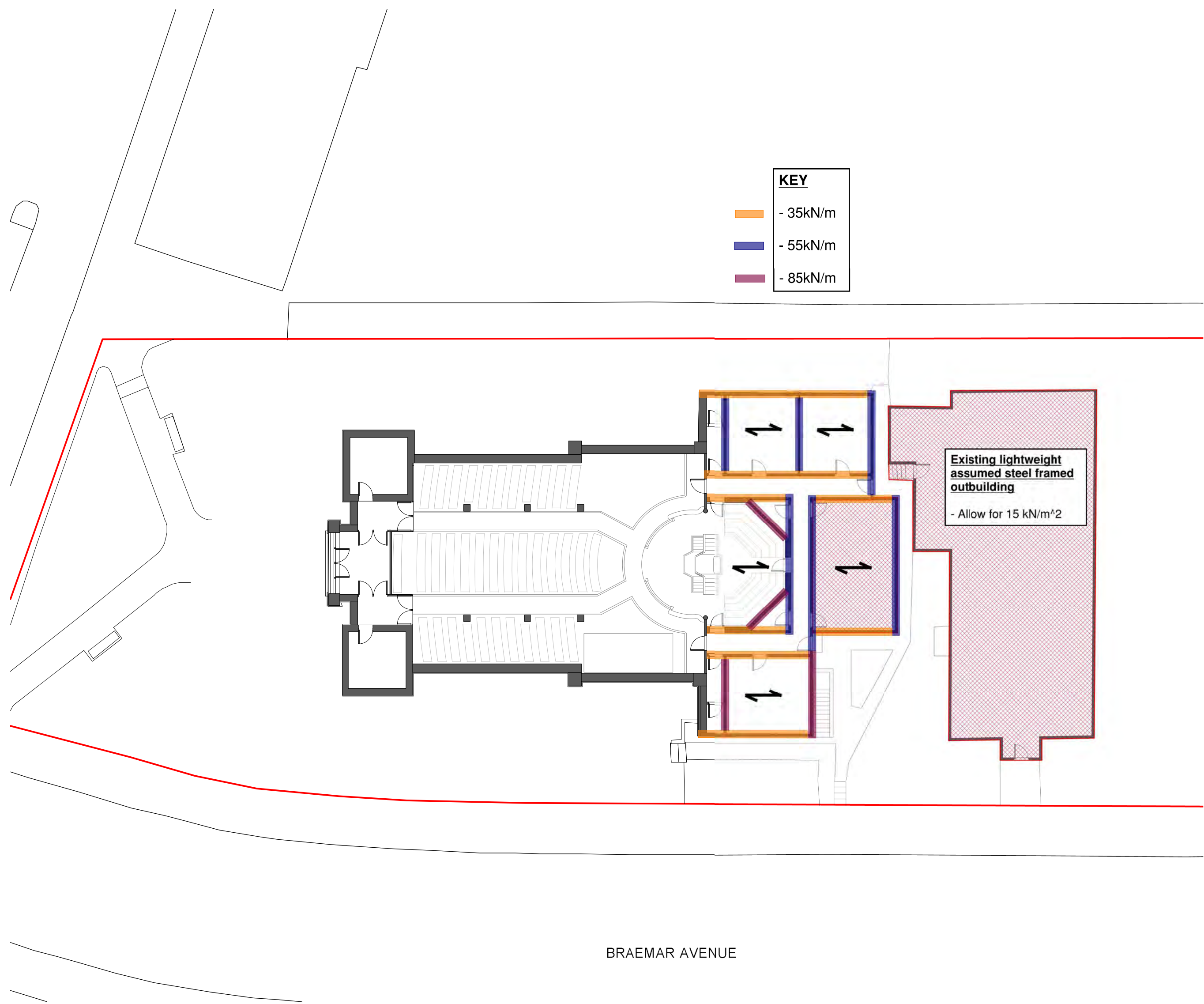
P1	07/12/22	TU	JCa	Revised structural scheme
rev	date	by	chk	description

Drawing title Proposed superstructure loads		
scale (s)	date	drawn
NTS	December 2022	TU

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Project Braemar Avenue, Wood Green				
Drawing status Preliminary		Status Revision S2 P1		
Project no.	Originator Zone	Level	Type	Role
2220367-EWP-ZZ-B1-SK	-	-	-	-



This drawing is to be read in conjunction with all relevant architects, engineers and specialists drawings and specifications.

Do not scale from this drawing.

- Notes**
- Indicative sketches for preliminary advice only.
 - Any changes to the architectural scheme will necessitate rechecking the structural scheme to ensure that superstructure loads are reflected accordingly.
 - **All loads are split into dead, live, wind and are given as unfactored.**
 - Foundations are assumed to be an RC raft foundations - TBC by geotechnical engineer.

P1	18/11/22	TU	JCa	Preliminary loads
rev	date	by	chk	description

Drawing title

Existing superstructure loads

scale (s)	date	drawn
NTS	November 2022	TU

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Project

Braemar Avenue, Wood Green

Drawing status

Preliminary

Status Revision

S2 P1

Project no.

2220367-EWP-ZZ-XX-SK-S-0002

Annex C: Desk Study & Site Walkover Data

- Envirocheck Report, ref.no.303877391_1_1, Landmark, 14/11/2022
- BGS Logs
- Site walkover description and associated plates

BGS historical borehole with reference no. TQ39SW651 (c.450m NW)

Contract Name HARINGEY						Borehole No. 1	
						Sheet 1 of 2	
Method of boring Light cable percussion boring				Ground level		3013 9111	
Diameter 200 mm nominal				Start 2.7.82			
				Finish 5.7.82			

Daily progress	Water levels	In-situ tests	Sam- ples	Depth (m)	Reduced level (m O.D.)	Thickness (m)	Description of Strata	Legend
			J	0.30		0.30	Topsoil	
			J	0.55		0.25	Light brown clay with traces of brick	
			J			1.70	Firm becoming stiff brown silty CLAY with occasional small pockets and partings of orange-brown fine sand and silts some root fibres and calcareous nodules	   
			J					
			U					
			U					
2/7				2.25				
			JU			8.75	Stiff becoming very stiff brown fissured silty CLAY with small pockets and partings of brown fine sand and silts, occasional roots to 5 m and pockets of selenite crystals. Claystone bands between 5.00 m to 5.30 m and 9.50 m to 9.60 m.	                
			U					
			U					
			U					
			JU					
			J					
			U					
			J					
			U					
			J					
			U					
			JU					

Notes
Clearing buried services 2 hours
7.00 m Water level observation tubing installed.

Contd/..

Terresearch Limited	Report No. S.32/545	Appendix 1 Sheet 1
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BGS historical boreholes with reference no. TQ39SW651 (c.450m NW)

Contract Name HARINGEY						Borehole No. 1		
						Sheet 2 of 2		
Method of boring				Ground level				
Diameter				Start				
				Finish				
Daily progress	Water levels	In-situ tests	Samples	Depth (m)	Reduced level (m Q.D.)	Thickness (m)	Description of Strata	Legend
			J	11.00				
			J					
2/7	5/7		U			4.00	Very stiff grey-brown fissured silty CLAY with small pockets and partings of grey fine sand and silt	
			J					
			U					
			J					
			U					
5/7				15.00				
							Bottom of Borehole	
Notes								
Terresearch Limited			Report No. S.32/545			Appendix 1 Sheet 2		

BGS historical borehole with reference no.TQ39SW355 (c.450m SE)



256/128
TQ 39/22

British Geological Survey

British Geological Survey

British Geological Survey

256/128

London Clay
Woolwich + Reading Beds (?Reading Type)
Thanet Sand
Upper Chalk

	ft	m
a. 141	6	
a. 48	6	

Stratigraphic Column 1976

British Geological Survey

British Geological Survey

British Geological Survey

British Geological Survey

British Geological Survey

British Geological Survey

British Geological Survey

British Geological Survey

British Geological Survey